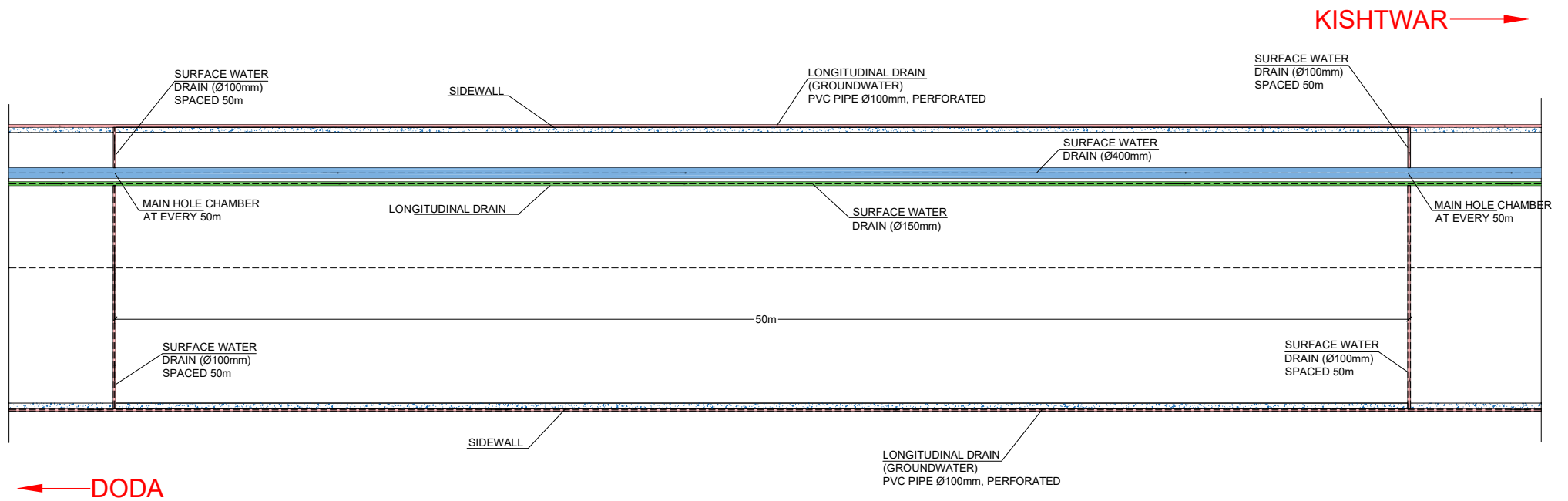
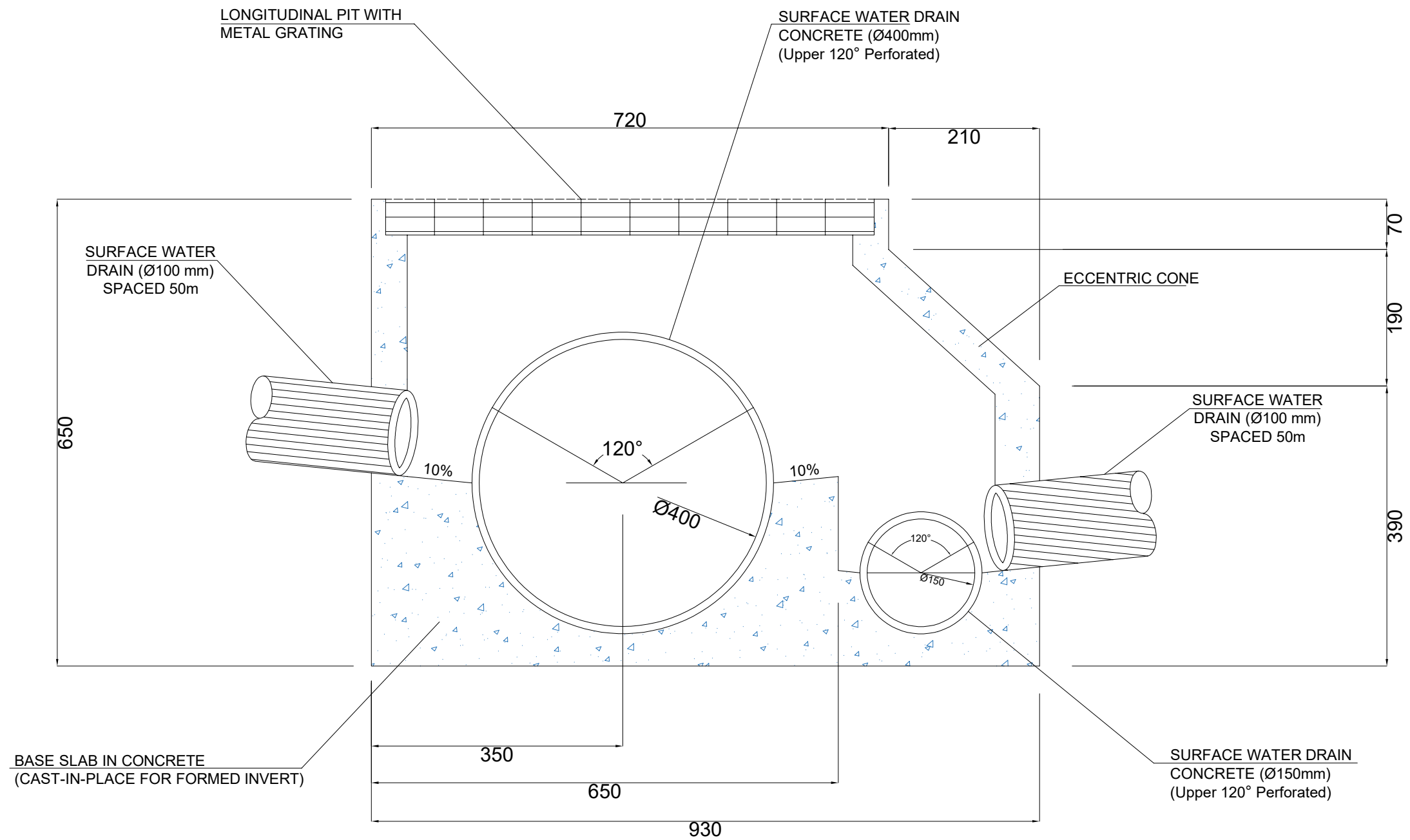




DETAIL PLAN



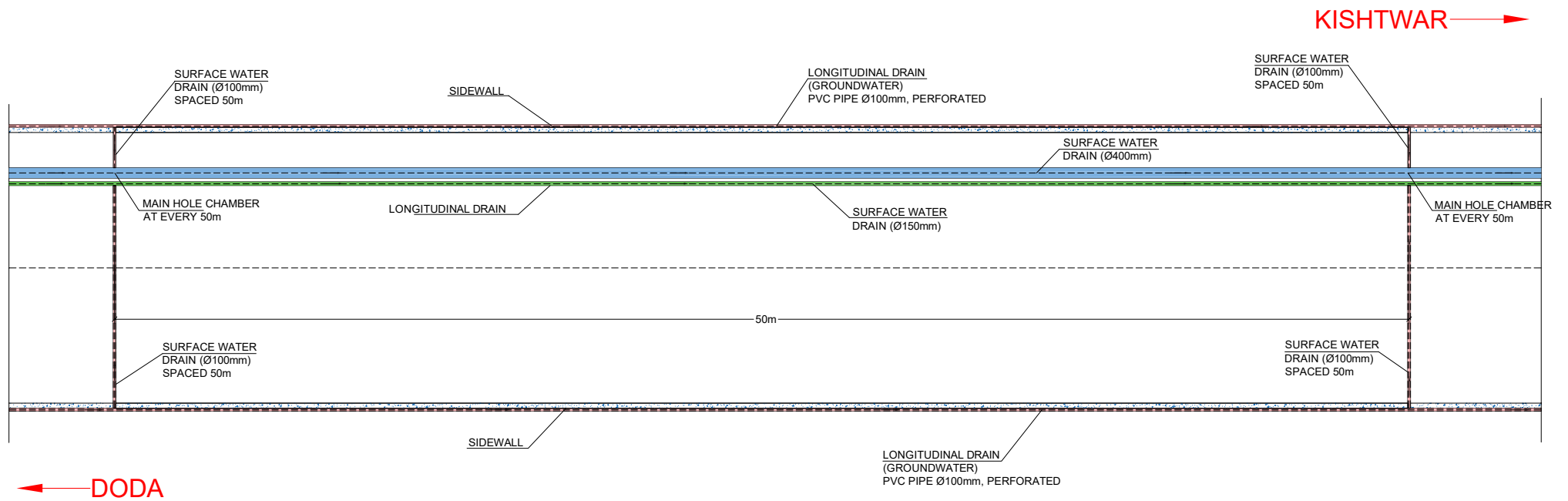
CLIENT  National Highways and Infrastructure Development Corporation PTI Building, 4 Parliament Street, Sansad Marg Area, New Delhi, Delhi 110001	PROJECT Construction Of Bi Directional tunnel at km 83 of length 495m including approaches road of 664 m and a major bridge of 110 m from km 82 +675 to km 82 +925 on NH- 244	EPC CONSULTANT  Belgh Construction Company Pvt.Ltd Plot no 4, Wave mall, Bypass road, Narwal Jammu email. info@bccgroupin	DESIGN CONSULTANT  Space Engineers Consortium Pvt.Ltd In associated with Equipo De Prospecciones ,S.A	<table><tr><td>R1</td><td>02-02-22</td><td rowspan="3">FINAL DESIGN</td></tr><tr><td>R0</td><td>22-06-21</td></tr><tr><td>REVISION</td><td>DATE</td></tr></table>	R1	02-02-22	FINAL DESIGN	R0	22-06-21	REVISION	DATE	DRAWN BY: CARMEN PEREZ SERRANO DESIGNED BY: DIANA CASTANEDO DE LUCAS CHECKED BY: PROF.FCO.CASTENEDO NAVARRO RECOMMENDED BY: DIANA CASTANEDO LUCAS APPROVED BY: PROF.FCO.CASTENEDO NAVARRO SCALE : 1:50	TITLE TUNNEL DRAINAGE SYSTEM TYPICAL CROSS SECTION DWG NO. KDBT/DREINND/R1 REVISION R1
R1	02-02-22	FINAL DESIGN											
R0	22-06-21												
REVISION	DATE												



DETAIL "A"
DIMENSIONS IN mm



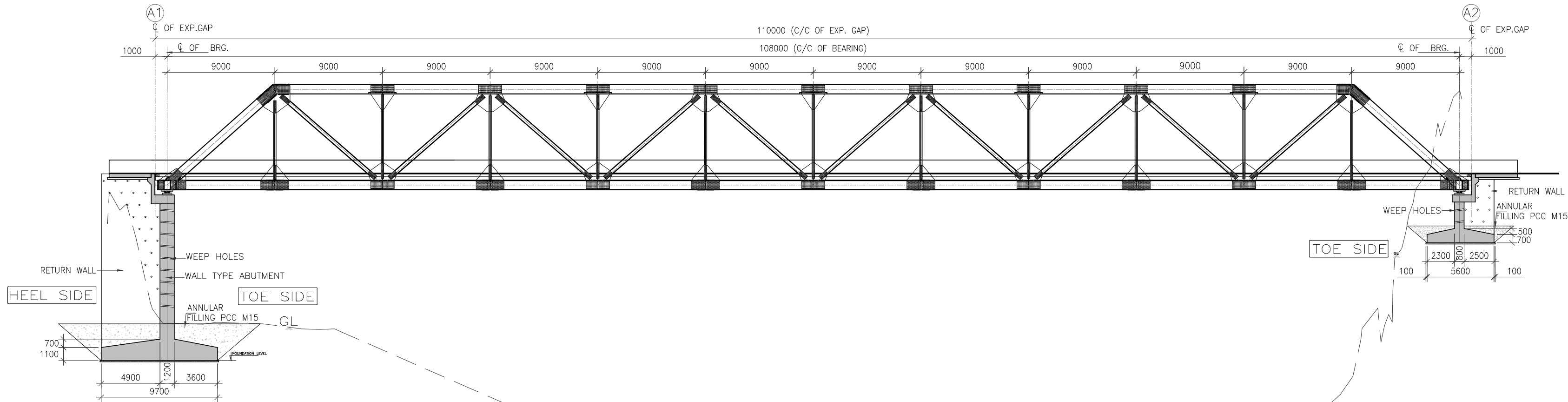
CLIENT		PROJECT		EPC CONSULTANT		DESIGN CONSULTANT				DRAWN BY : CARMEN PEREZ SERRANO		TITLE			
 National Highways and Infrastructure Development Corporation PTI Building ,4 Parliament Street,Sansad Marg Area, New Delhi,Delhi 110001		Construction Of Bi Directional tunnel at km 83 of length 495m including approaches road of 664 m and a major bridge of 110 m from km 82 +675 to km 82 +925 on NH- 244		 Belgh Construction Company Pvt,Ltd Plot no 4, Wave mall,Bypass road, Narwal Jammu email. info@bccgroupin		 Space Engineers Consortium Pvt,Ltd In associated with Equipo De Prospecciones ,S.A				R1 02-02-22 FINAL DESIGN R0 22-06-21 FINAL DESIGN REVISION DATE DESCRIPTION		DESIGNED BY : DIANA CASTANEDO DE LUCAS CHECKED BY : PROF.FCO.CASTENEDO NAVARRO RECOMMENDED BY : DIANA CASTANEDO LUCAS APPROVED BY : PROF.FCO.CASTENEDO NAVARRO SCALE : 1:50		TUNNEL DRAINAGE SYSTEM TYPICAL CROSS SECTION DRG NO. KDBT/DREIN/IND/R1 REVISION R1	



DETAIL PLAN

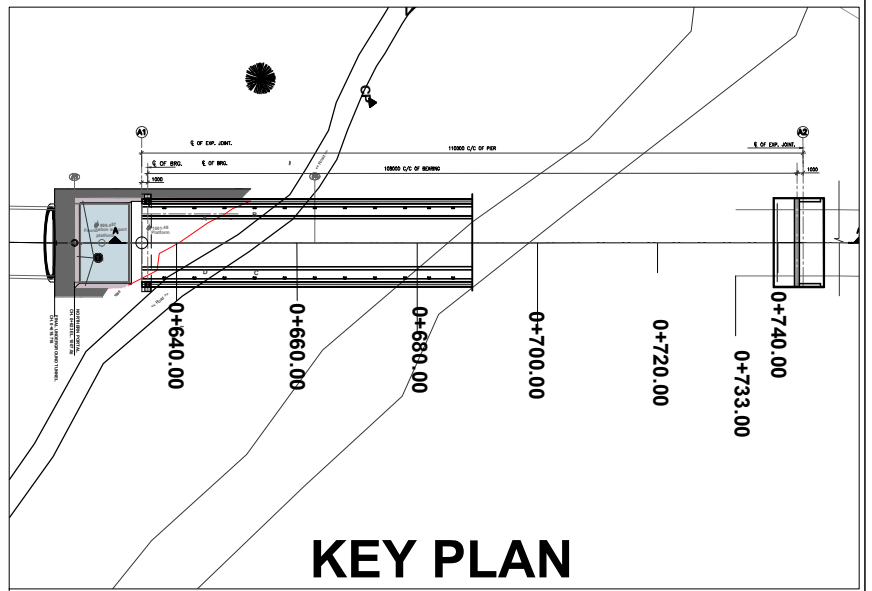
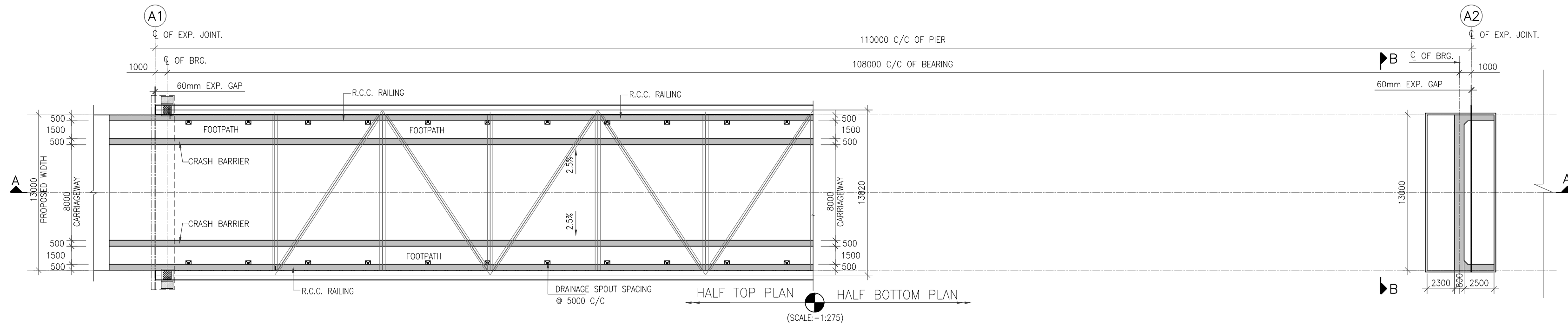


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R1	02-02-22	FINAL DESIGN													
R0	22-06-21	FINAL DESIGN													
REVISION	DATE	DESCRIPTION													



SECTION A-A
(SCALE:-1:275)

PROPOSED LVL AT LME	1007.199	1007.112
GROUND LEVELS	994.444	1004.112
PROPOSED CHAINAGE	0+634	0+744



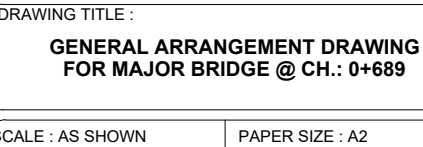
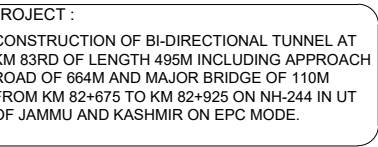
KEY PLAN

NOTES:-

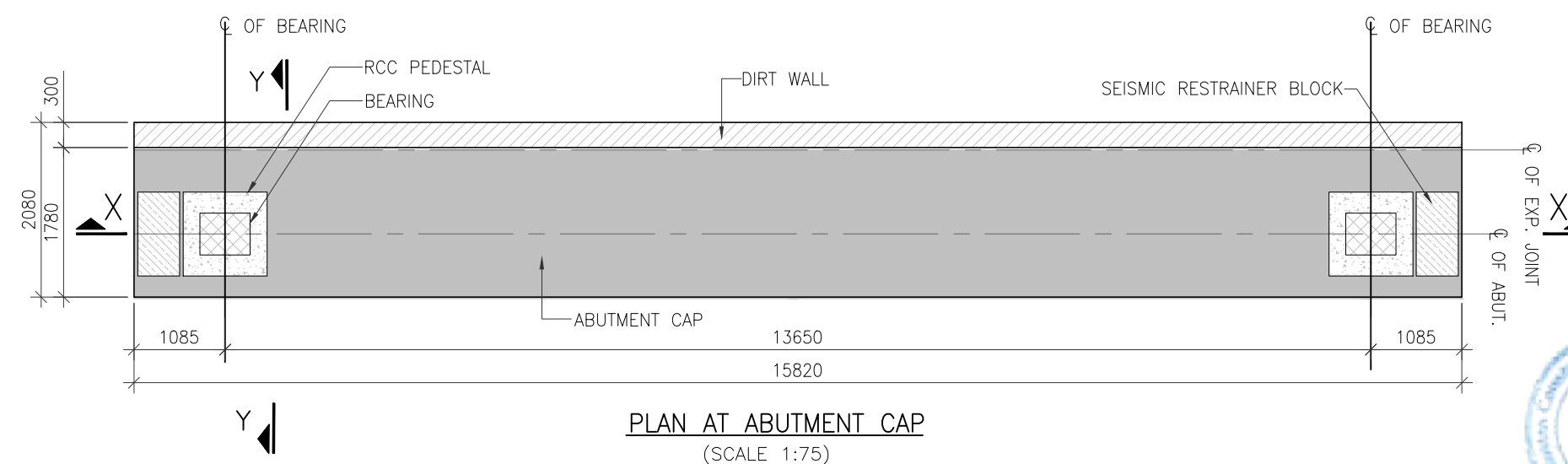
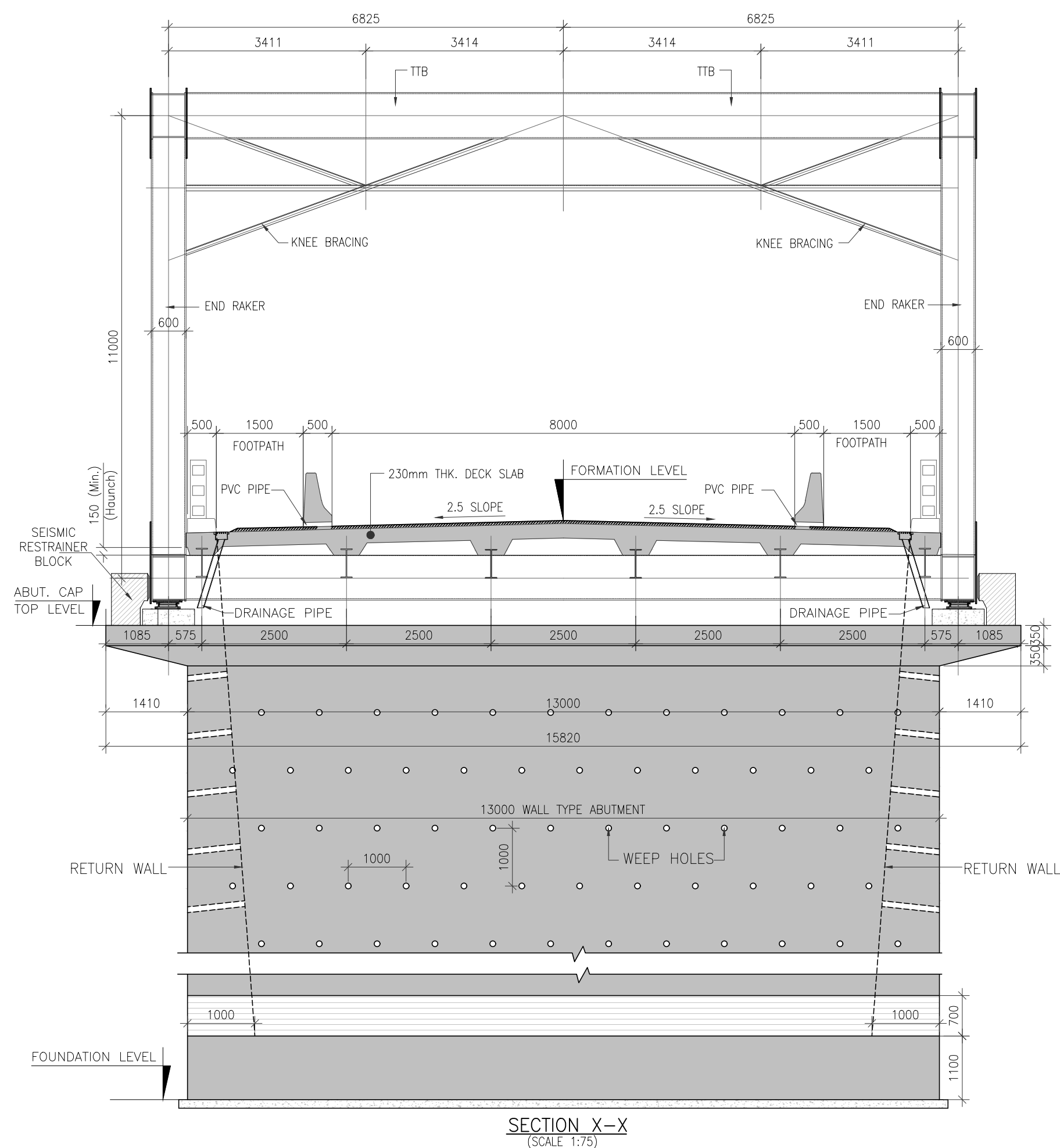
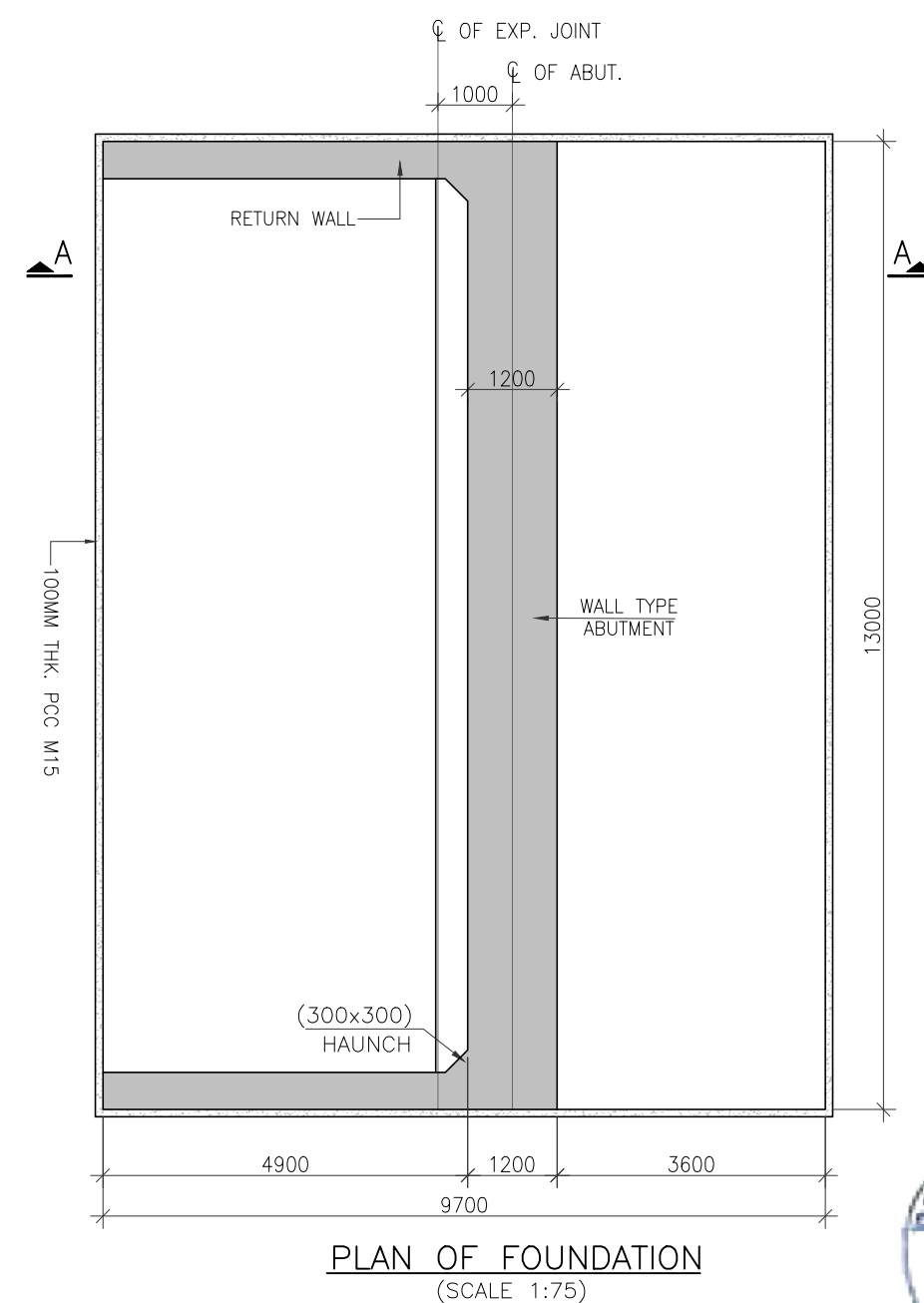
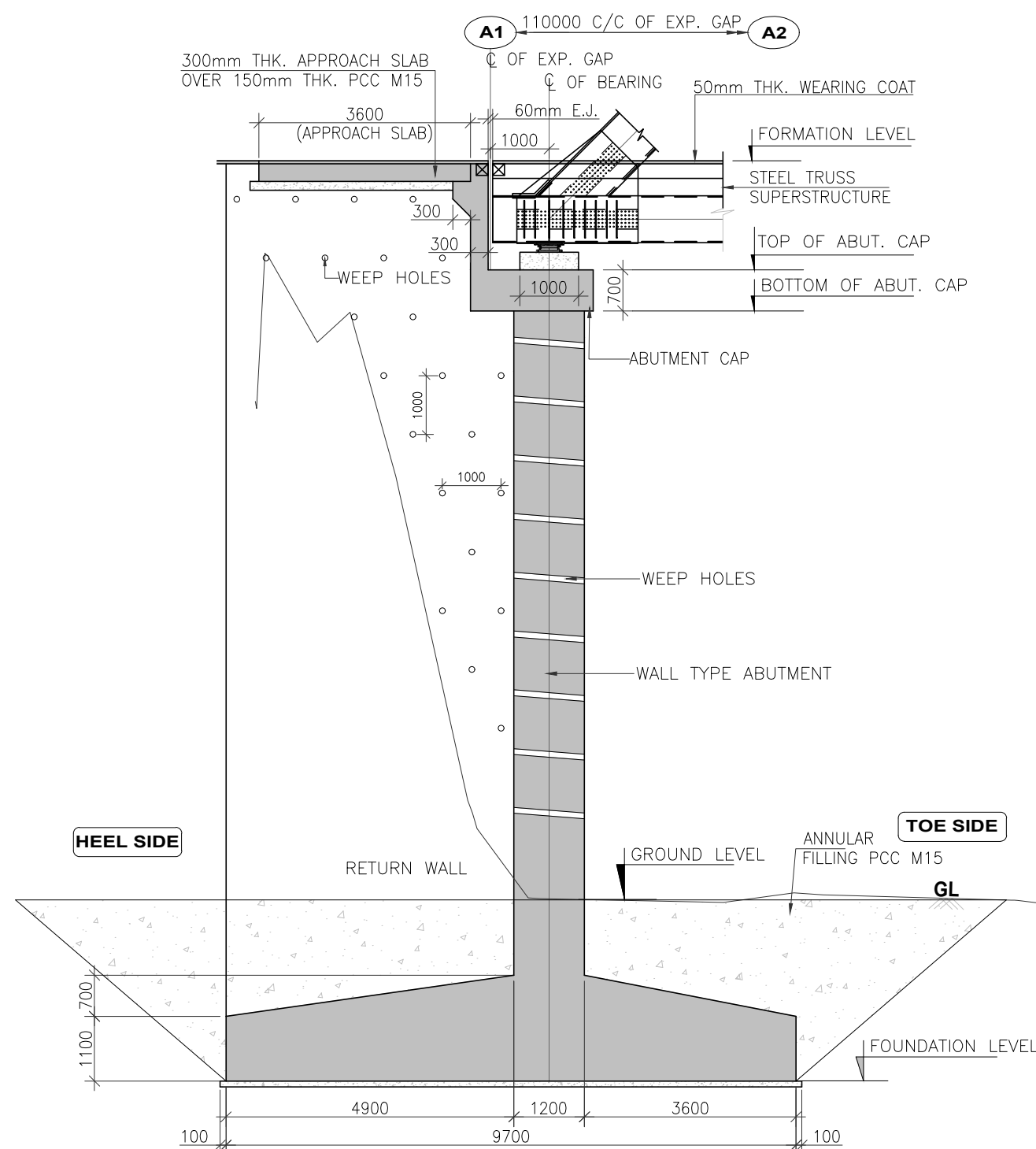
- ALL DIMENSIONS ARE IN MM AND LEVELS ARE IN METERS UNLESS OTHERWISE MENTIONED.
- DIMENSIONS ARE NOT TO BE SCALED. ONLY WRITTEN DIMENSIONS ARE TO BE FOLLOWED.
- DURING WELDING, POSITION OF MEMBERS AND ASSEMBLY SHOULD BE SUCH THAT OVERHEAD WELDING SHALL BE AVOIDED/MINIMIZED.
- ALL THE MEMBERS MEETING AT A JOINTS SHOULD BE WELDED AROUND.
- ALL WELDING ELECTRODES FOR 'GMAW' PROCESS SHALL CONFORM TO AWS A 505.
- ALL BARE ELECTRODES FOR 'GMAW' PROCESS SHALL CONFORM TO AWS A SUBMERGED ARC WELDING (SAW) SHALL BE AS PER AWS A 5.23.
- ALL WELDING TO BE DONE IN CONFORMITY WITH IS:9595 & OR AWS D 1.1/D 1.5 & OTHER RELEVANT SPECIFICATION.
- BREAKUP OF DIMENSIONS SHOULD TALLY WITH OVERALL DIMENSION FOR ALL THE MEMBERS.
- STRIP SEAL EXPANSION JOINTS SHALL BE PROVIDED ON EXPANSION END.
- BEARINGS SHALL BE POT/PTFE BEARINGS.
- GRADE OF DECK SLAB IS M-40.
- THE DESIGN IS IN ACCORDANCE WITH IRC:5-2015, IRC:6-2017, IRC:112-2011 & IRC:24-2010.
- HIGH STRENGTH FRICTION GRIP BOLT (HSFG) OF GRADE (10.9) MAY BE USED IN CONFORMITY WITH IS:4000- 1992. SLIP FACTOR SHOULD BE TAKEN 0.35.
- FOR LIFTING OF SPAN IN FUTER, JACKS WELL BE LOCATED UNDER END CROSS GIRDERS AT THE LOCATION SHOWN.
- WHEN THE SPAN IS LIFTED ON JACKS LIVE LOAD WILL NOT BE FOLLOWED OVER THE BRIDGE, JACK REQUIRED FOR LIFTING OF SPAN SHALL HAVE MINIMUM CAPACITY OF 250 TON
- DO NOT PROVIDE ANY PAINT SUCH AS RED OXIDE, PRIMER OR ANY OTHER COATING OVER HSFG BOLT CONNECTION AREA



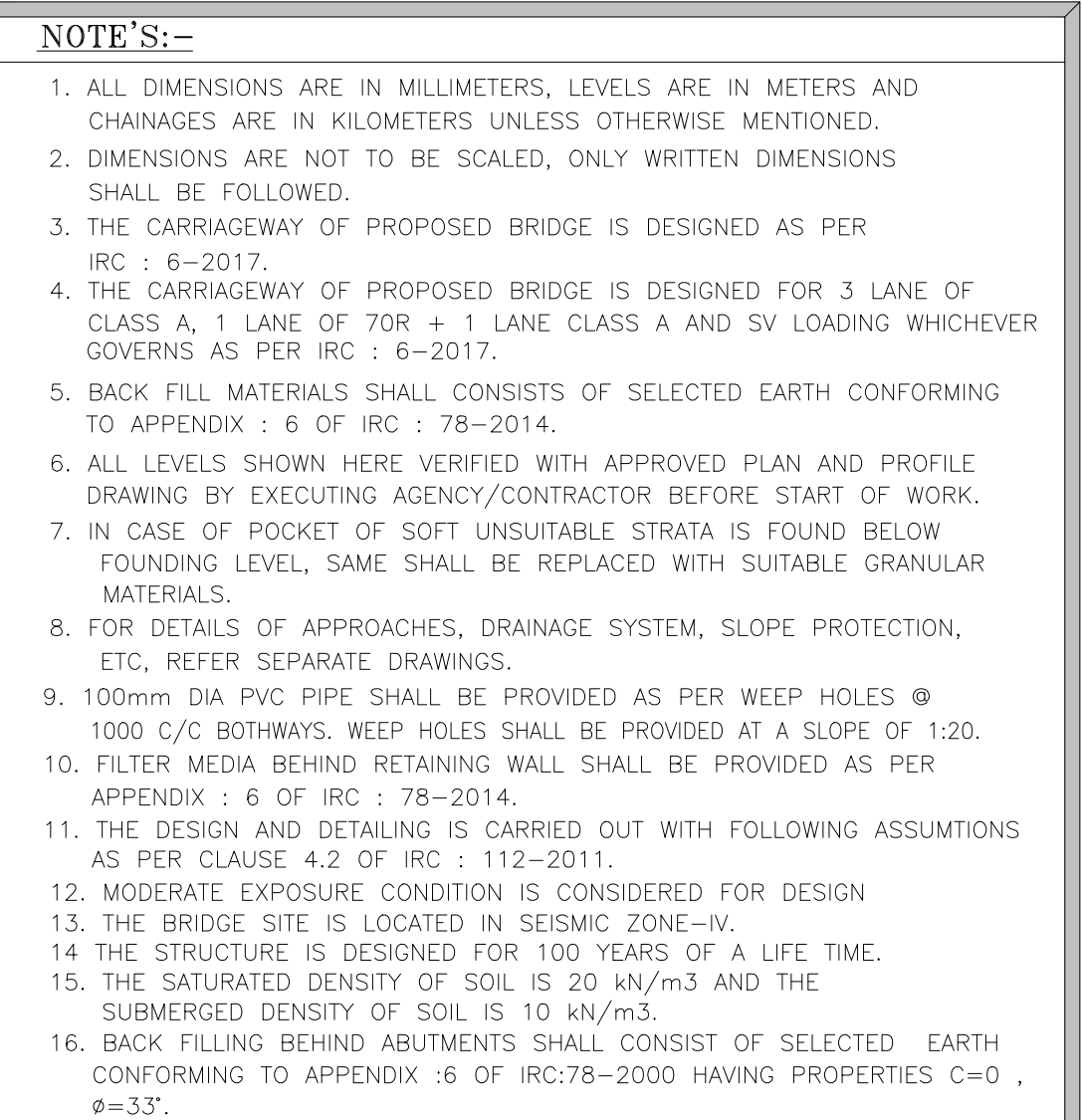
REV	DATE	DESCRIPTION	A	B	C	D
R0	SEP-2022	SUBMISSION OF MJB				



DRG. NO. STR/DWG/MJB/GAD/101	DATE : SEPTEMBER. 2022	REV: R0
DESIGNED GS	DRAWN SP	CHECKED AS
APPROVED SM		



ABUTMENT LEVELS						
LOCATION	FINISHED ROAD LEVEL	ABUTMENT CAP TOP LEVEL	ABUTMENT CAP BOTTOM LEVEL	GROUND LEVEL	FOUNDATION TOP LEVEL	FOUNDATION BOTTOM LEVEL
A1	1007.199	1004.984	1004.284	994.444	994.144	992.344



GRADES, MATERIAL TYPES & ETC.:-

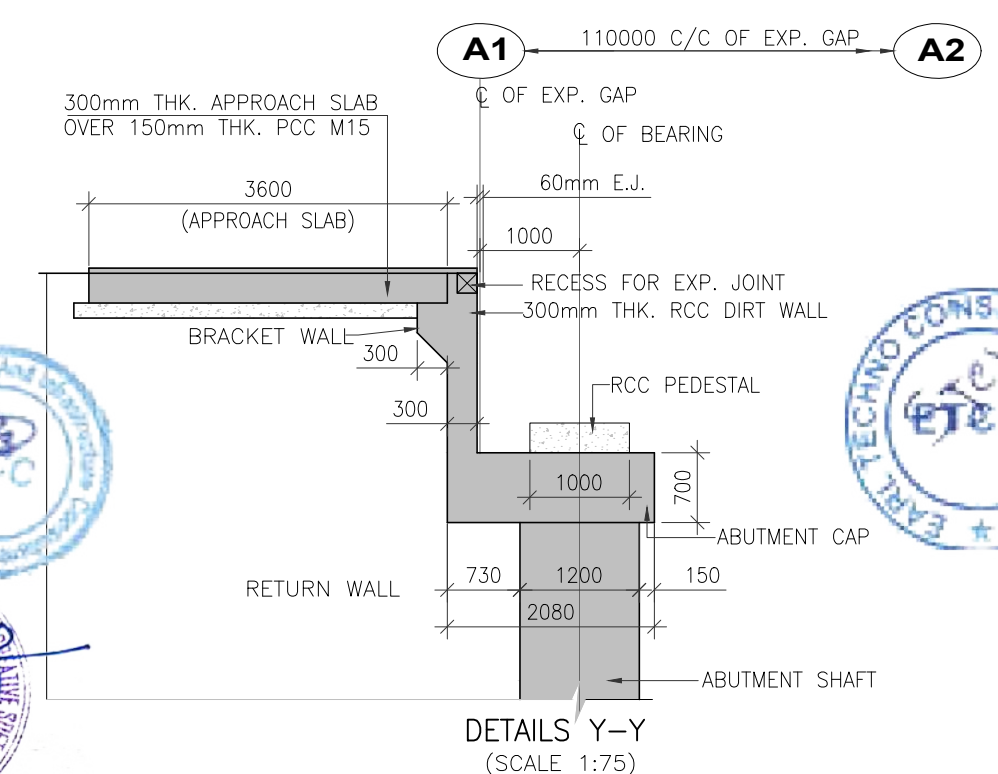
- | | |
|--|-----------|
| a) RCC CRASH BARRIER | M40 |
| b) SUBSTRUCTURE / FOUNDATION..... | M35 |
| c) APPROACH SLAB | M30 |
| d) ABUTMENT CAP | M35 |
| e) PEDESTAL..... | M40 |
| f) PCC | M15 |
| g) SEISMIC ZONE..... | (IV) |
| h) B.C WEARING COAT..... | 50MM THK. |
| i) STEEL/HYSD BARS (CONFIRM TO IS:1786)..... | Fe500D. |

CLEAR COVER:-

- | | |
|---|------|
| a) RCC CRASH BARRIER | 50mm |
| b) FOUNDATION | 75mm |
| c) RCC ABUTMENT SHAFT (EARTH FACE) | 75mm |
| d) RCC ABUTMENT SHAFT (NON-EARTH FACE)..... | 40mm |
| e) ABUTMENT CAP..... | 50mm |
| f) DIRT WALL | 50mm |
| g) APPROACH SLAB | 50mm |

SPECIAL NOTES:

- 01 "SAFE BEARING PRESSURE CONSIDERED IN DESIGN IS 70 T/m² AND THE SAME
SHALL BE VERIFIED AT SITE BEFORE EXECUTION OF WORK."
02 "PLATE LOAD TEST SHALL BE DONE AT SITE."



R0	SEP-2022	SUBMISSION OF MJB					
REV	DATE	DESCRIPTION	A	B	C	D	

CLIENT:



NATIONAL HIGHWAYS AND INFRASTRUCTURE
DEVELOPMENT CORPORATION LTD.

AUTHORITY: AIAPL

EPC CONTRACTOR :

**BCC**
BEIGH CONSTRUCTION CO. PVT. LTD.
Partners in Progress

BEIGH CONSTRUCTION COMPANY PVT. LTD

PROOF CONSULTANT :
HCC
HINDUSTAN CONSTRUCTION &
INFRASTRUCTURE CONSULTANTS

SAFETY CONSULTANT :

**EARL TECHNO
CONSULTANTS PVT. LTD.**

EARLTECHNO CONSULTANTS PVT. LTD

DESIGN CONSULTANT :

 FUTURE BUILD COMPANY

PROJECT :
CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT
KM 83RD OF LENGTH 495M INCLUDING APPROACH
ROAD OF 664M AND MAJOR BRIDGE OF 110M
FROM KM 82+675 TO KM 82+925 ON NH-244 IN UT
OF JAMMU AND KASHMIR ON EPC MODE.

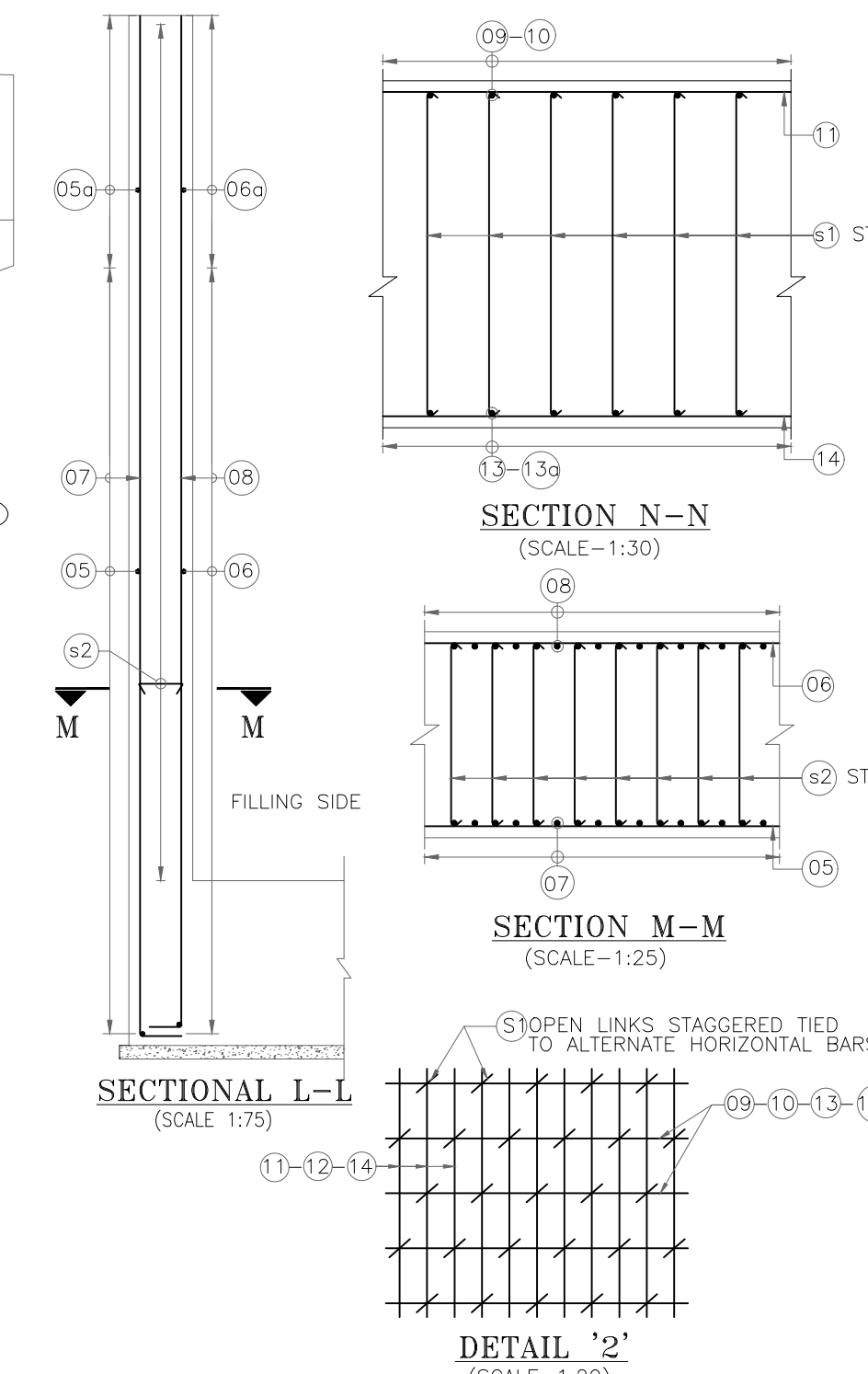
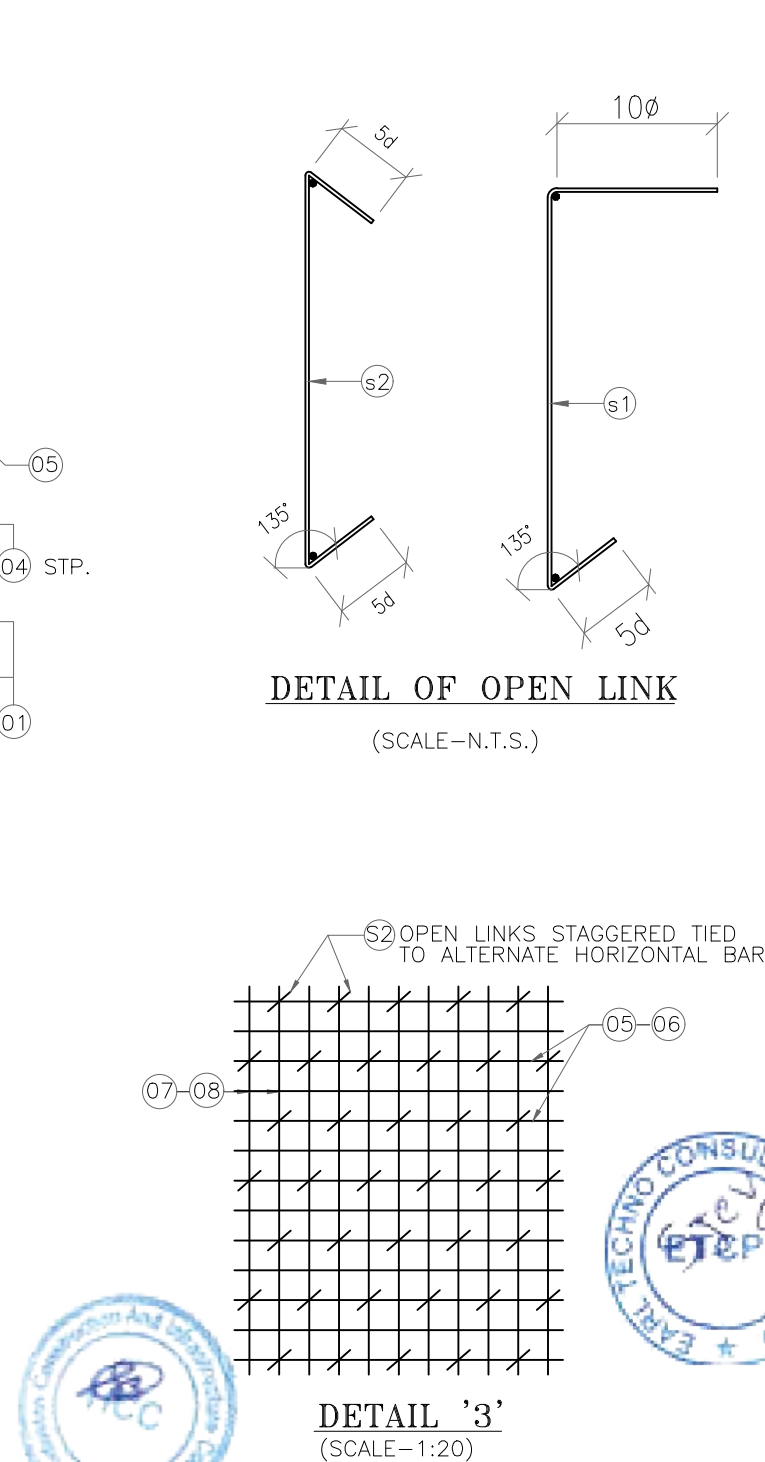
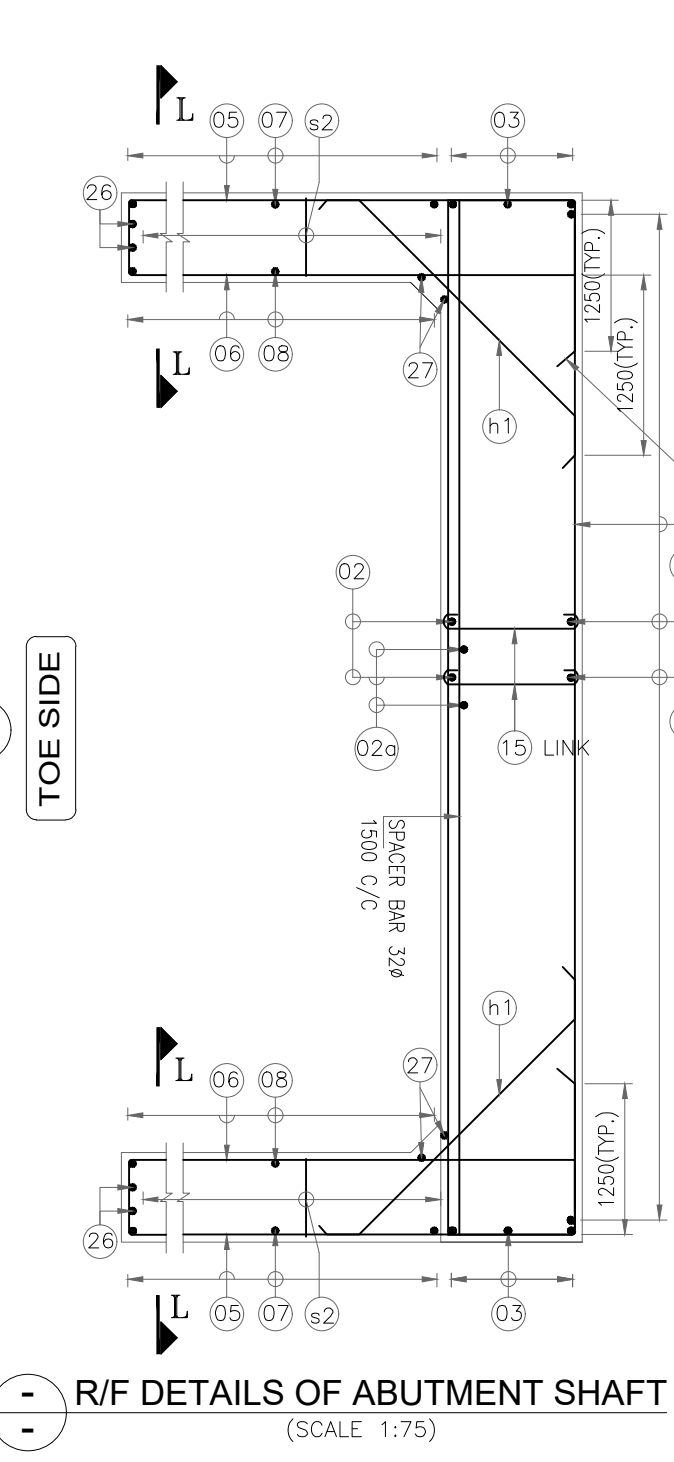
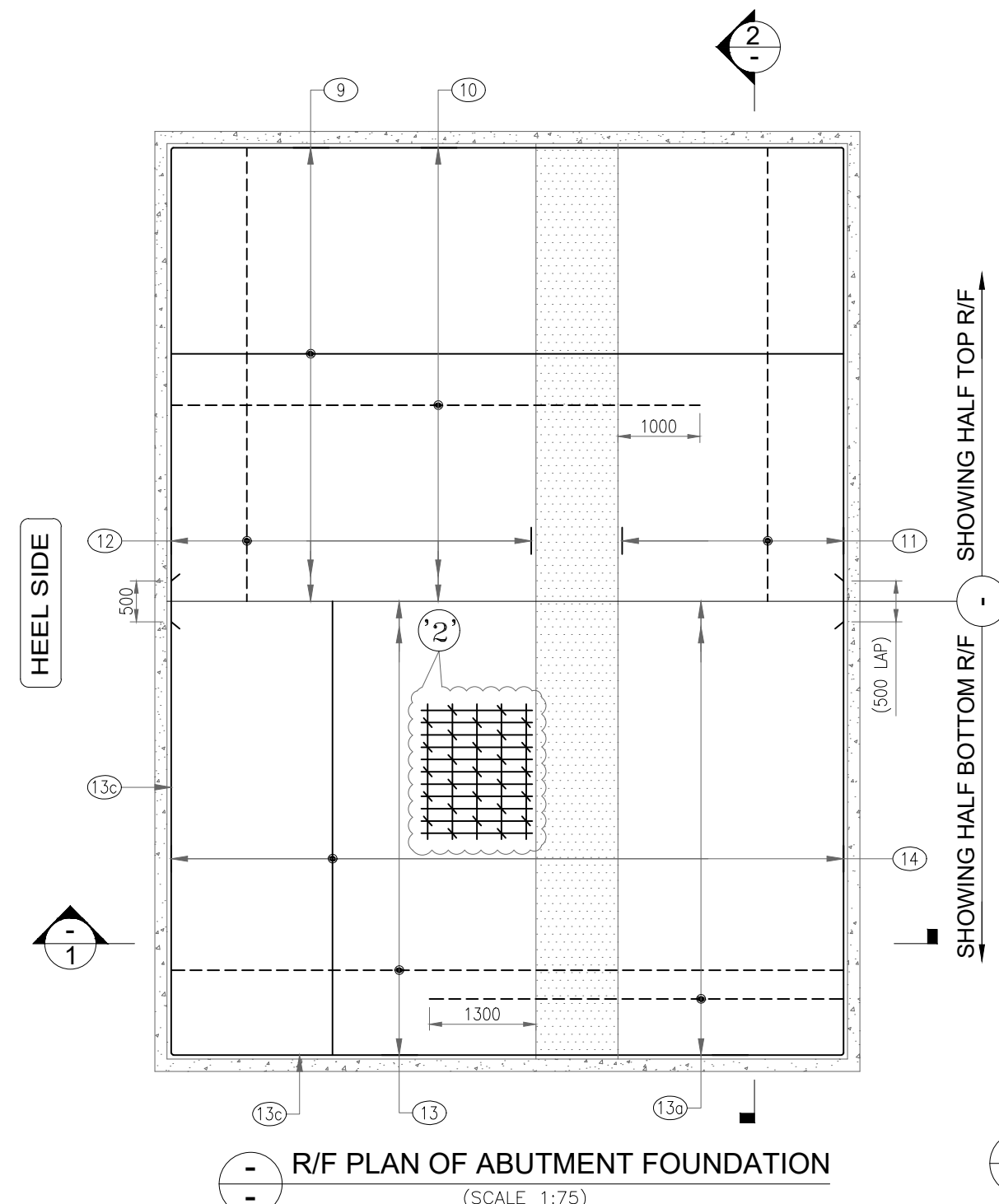
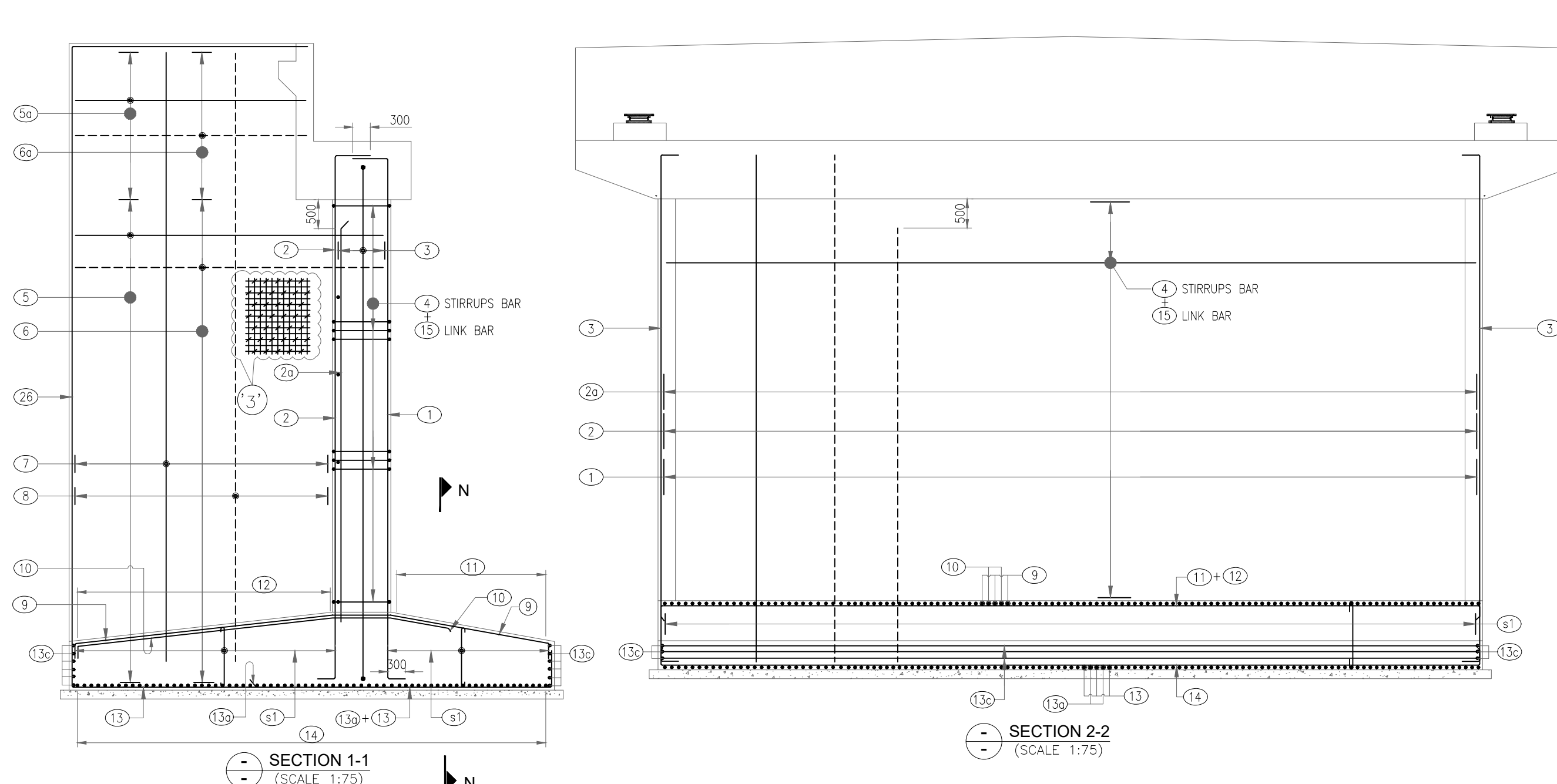
DRAWING TITLE :
DIMENSION DETAILS DRAWING OF
ABUTMENT (A1) FOR MJB AT CH. 0+689

SCALE : AS SHOWN PAPER SIZE : A2

DRG. NO. STR/DWG/MJB/DIM./ABUT.-A1/201			
DATE : SEPTEMBER. 2022			REV: R0
DESIGNED GS	DRAWN SP	CHECKED AS	APPROVED SM

DATE : SEPTEMBER, 2022	REV: R0
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DESIGNED GS	DRAWN SP	CHECKED AS	APPROVED SM
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REINFORCEMENT SCHEDULE FOR ABUTMENT-A1

BAR MKD.	SHAPE (NOT TO SCALE)	DIA. (mm)	TOTAL NOS. / SPACING (mm)
01		25	90 C/C
02		32	90 C/C
02a		32	90 C/C
03		20	100 C/C
04	PLAN	20	100 C/C
05		16	200 C/C
05a		16	150 C/C
06		25	150 C/C
06a		20	150 C/C
07		16	200 C/C
08		25	150 C/C
09		25	100 C/C
10		BAR MARK NOT USED	
11		16	150 C/C
12		16	150 C/C
13		25	100 C/C
13a		25	100 C/C
13b	BAR MARK NOT USED		
13c		16	100 C/C
14		20	150 C/C
15		10	200 C/C VERTICALLY
26		12	2 x 5 NOS.
h1		10	100 C/C
s1		10	OPEN LINK 200 C/C
s2		10	OPEN LINK 200 C/C
27		12	2 x 2 NOS.

NOTE'S:-

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- THE CARRIAGEWAY OF PROPOSED BRIDGE IS DESIGNED AS PER IRC : 6-2017.
- ADEQUATE LINKS SPACER BARS ETC SHALL BE PROVIDED FOR PROPER POSITIONING OF REINFORCEMENT.
- LAP JOINT SHALL BE PROVIDED IN STAGGERED MANNER & NOT MORE THAN 50% BAR SHALL BE LAPPED AT ANY SECTION.
- BOND LENGTH SHALL BE 50 TIMES DIA OF BAR & LAP LENGTH SHALL BE 73 TIMES DIA OF BAR.
- ABUT. CAP TOP LEVEL AND PEDESTAL HEIGHT MAY BE VARIES AFTER DESIGN OF BEARING SIZE.

GRADES, MATERIAL TYPES & ETC:-

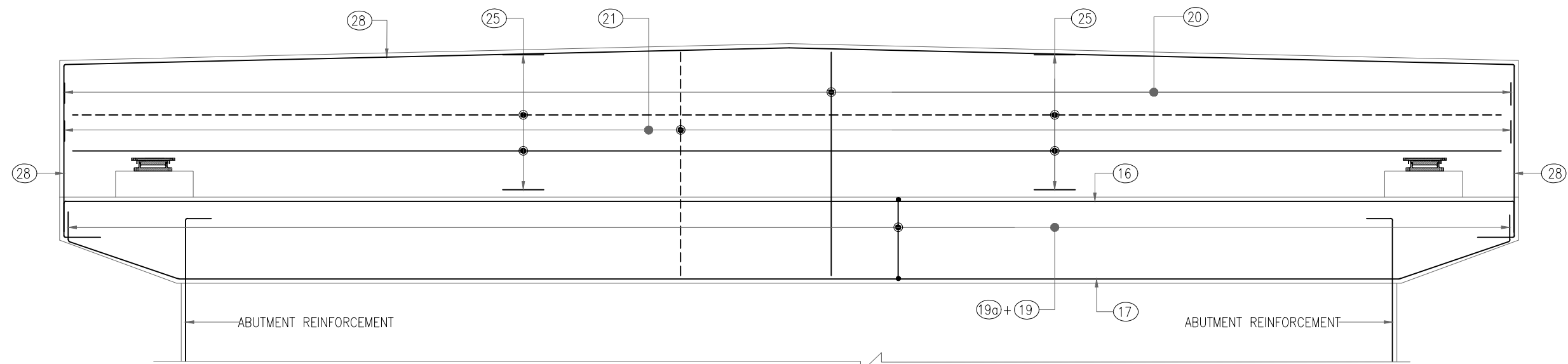
- RCC CRASH BARRIERM40
- SUBSTRUCTURE / FOUNDATION.....M35
- APPROACH SLABM30
- ABUTMENT CAPM35
- PEDESTAL.....M40
- PCCM15
- SEISMIC ZONE.....(IV)
- B.C WEARING COAT.....50MM THK.
- STEEL/HYSD BARS (CONFIRM TO IS:1786).....Fe500D.

CLEAR COVER:-

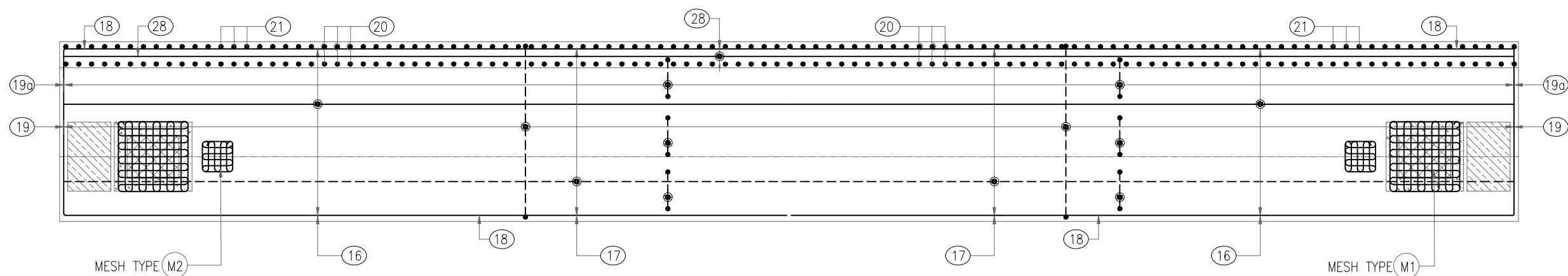
- RCC CRASH BARRIER50mm
- FOUNDATION75mm
- RCC ABUTMENT SHAFT (EARTH FACE)75mm
- RCC ABUTMENT SHAFT (NON-EARTH FACE).....40mm
- ABUTMENT CAP.....50mm
- DIRT WALL50mm
- APPROACH SLAB50mm

SPECIAL NOTES:

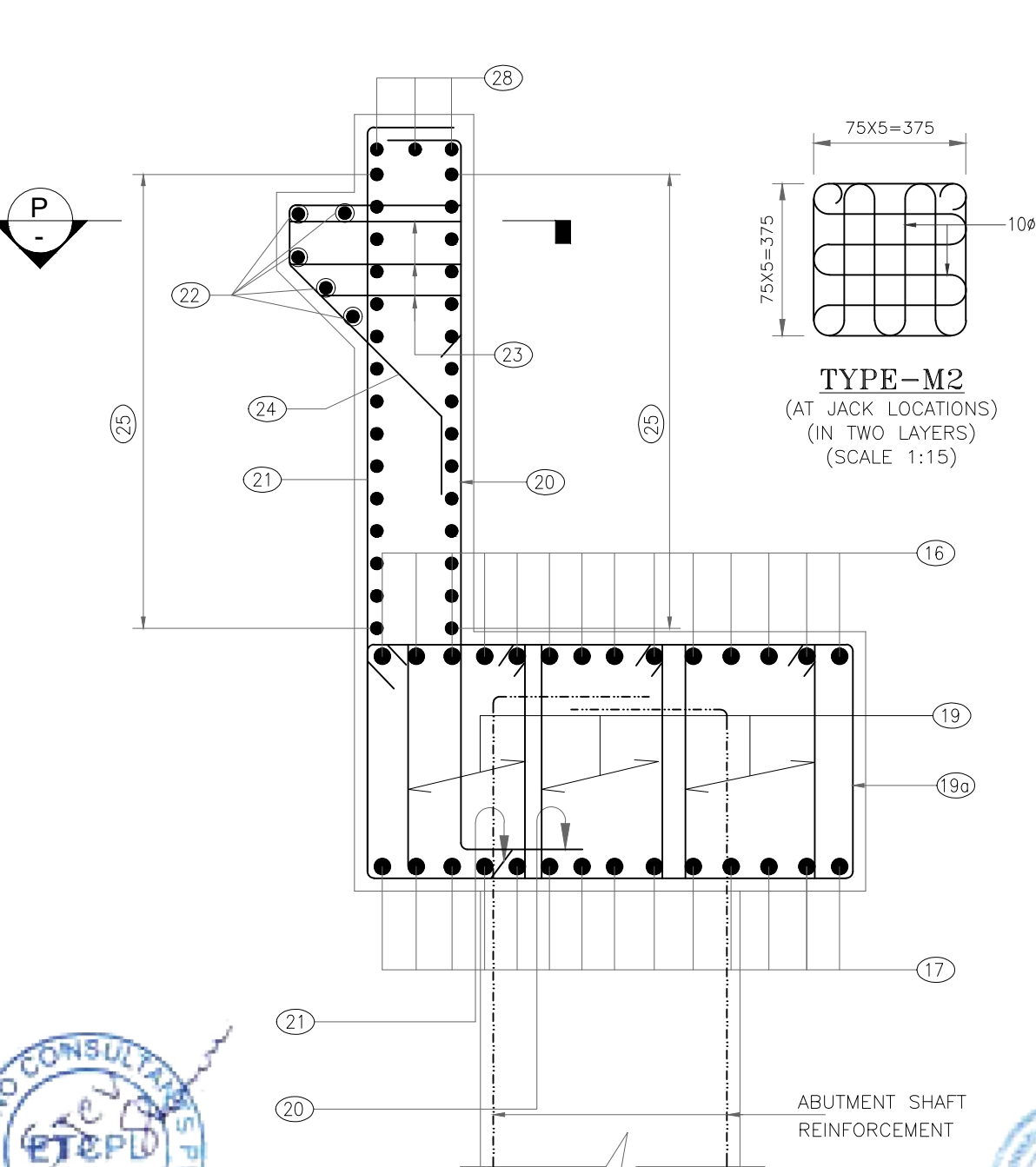
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- "PLATE LOAD TEST SHALL BE DONE AT SITE."



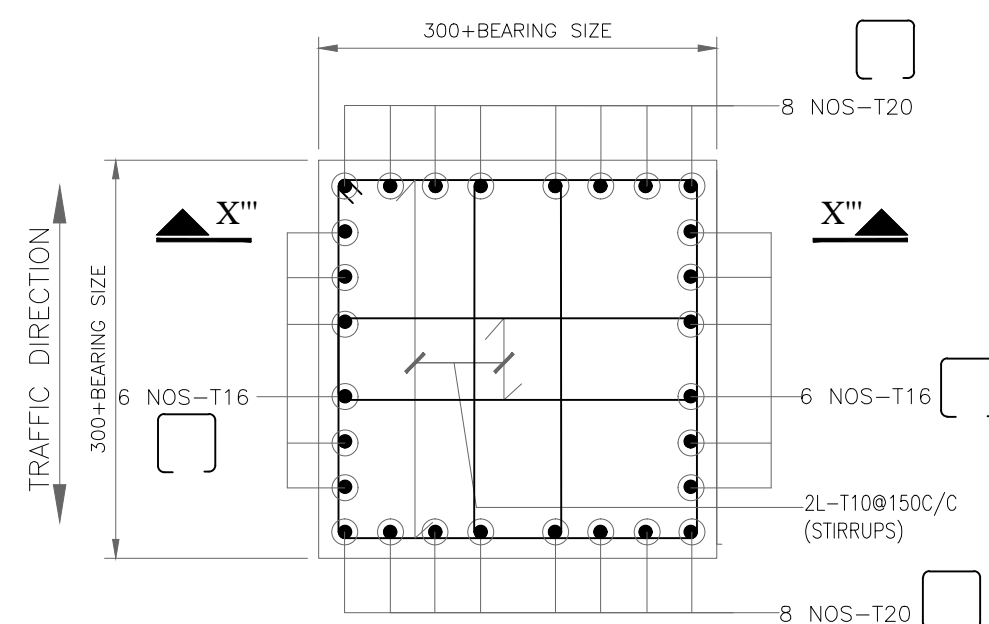
SECTION 2-2
(SCALE 1:50)



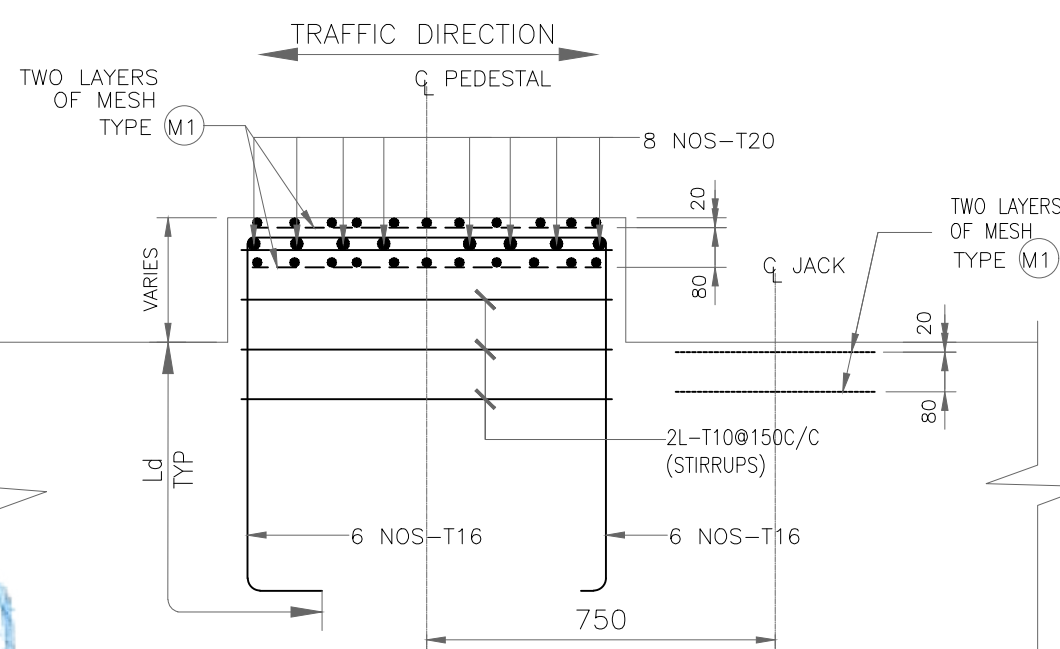
PLAN AT ABUTMENT CAP
(SCALE 1:75)



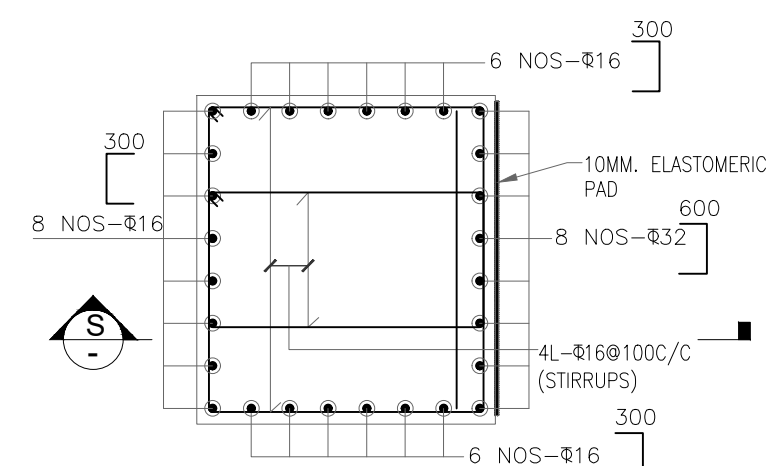
R/F DETAILS OF ABUT. CAP
(SCALE 1:40)



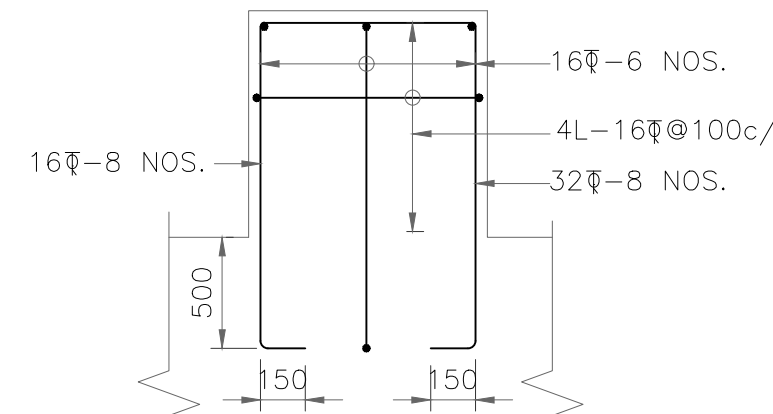
PLAN OF BEARING PEDESTAL
(SCALE 1:15)



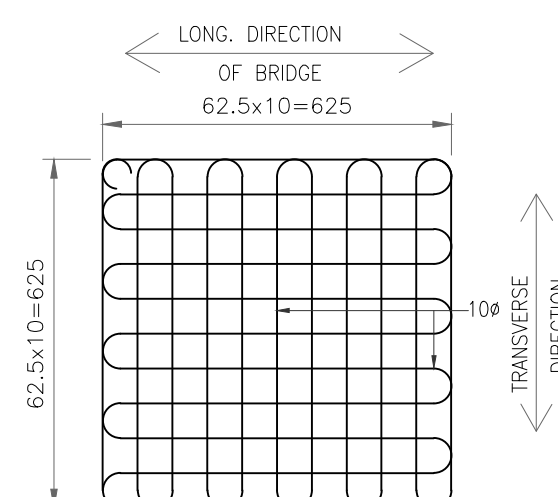
SECTION AT X'''- X'''
(SCALE 1:15)



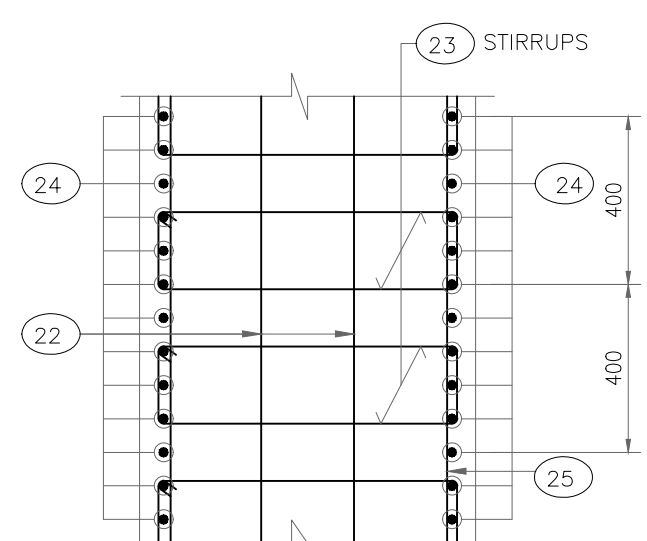
R/F DETAILS OF SEISMIC BLOCK
(SCALE 1:25)



SECTION S-S
(SCALE 1:30)



TYPE-M1
(AT BEARINGS LOCATIONS)
(IN TWO LAYERS)
(SCALE 1:15)



SECTION P-P
(SCALE 1:25)

REINF.SCHEDULE FOR ABUTMENT-A1

BAR MKD.	SHAPE (NOT TO SCALE)	DIA. (mm)	TOTAL NOS. / SPACING (mm)
16		16	120 C/C
17		16	120 C/C
18	NOT IN USE		
19		12	150 C/C
19a		12	150 C/C
20		12	120 C/C
21		12	120 C/C
22		10	6 NOS.
23		10	400 C/C 2 LEGGED 3 NOS.(HORI.)
24		12	100 C/C
25		10	150 C/C
28		16	3 NOS.

NOTE'S:-

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- ABUT. CAP TOP LEVEL AND PEDESTAL HEIGHT MAY BE VARIES AFTER DESIGN OF BEARING SIZE.

GRADES, MATERIAL TYPES & ETC.:-

- RCC CRASH BARRIERM40
- SUBSTRUCTURE / FOUNDATION.....M35
- APPROACH SLABM30
- ABUTMENT CAPM35
- PEDESTAL.....M40
- PCCM15
- SEISMIC ZONE.....(IV)
- B.C WEARING COAT.....50MM THK.
- STEEL/HYSD BARS (CONFIRM TO IS:1786).....Fe500D.

CLEAR COVER:-

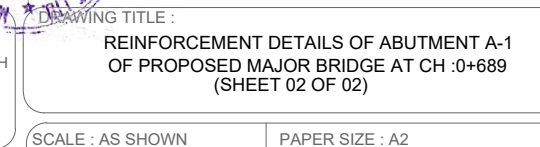
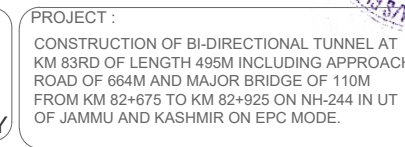
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- FOUNDATION75mm
- RCC ABUTMENT SHAFT (EARTH FACE)75mm
- RCC ABUTMENT SHAFT (NON-EARTH FACE).....40mm
- ABUTMENT CAP.....50mm
- DIRT WALL50mm
- APPROACH SLAB50mm

SPECIAL NOTES:

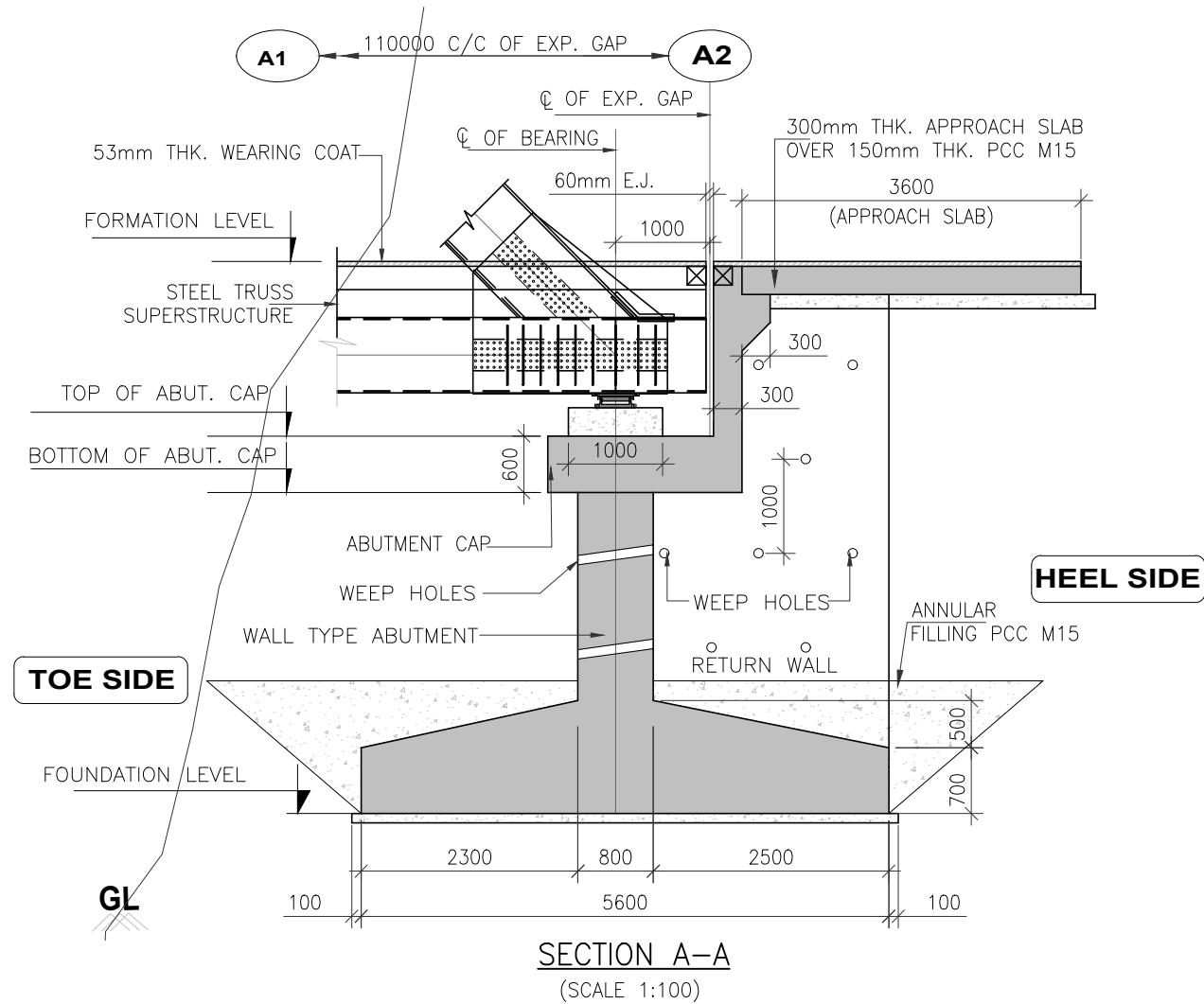
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- "PLATE LOAD TEST SHALL BE DONE AT SITE."



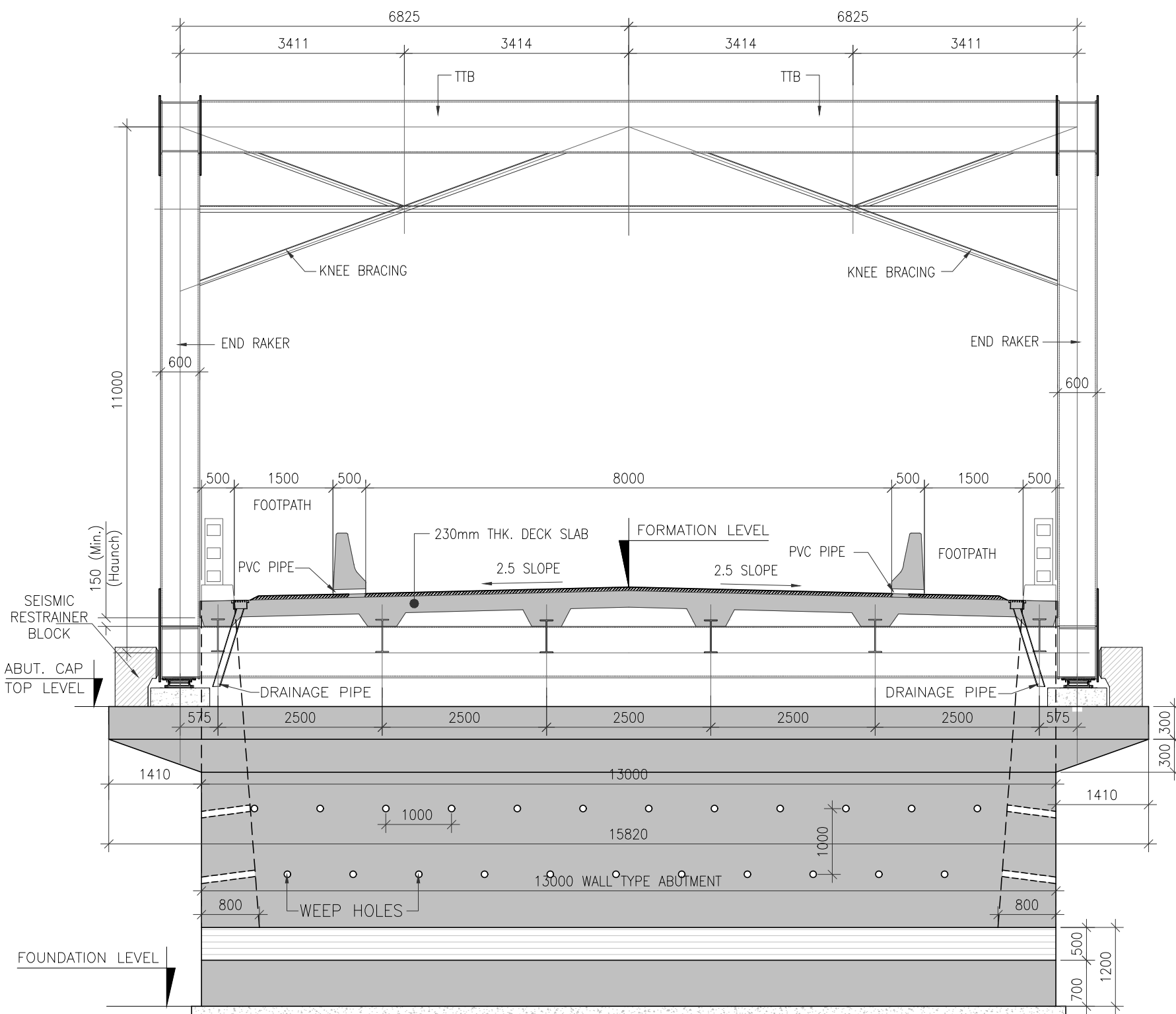
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R0	SEP-2022	SUBMISSION OF MJB				



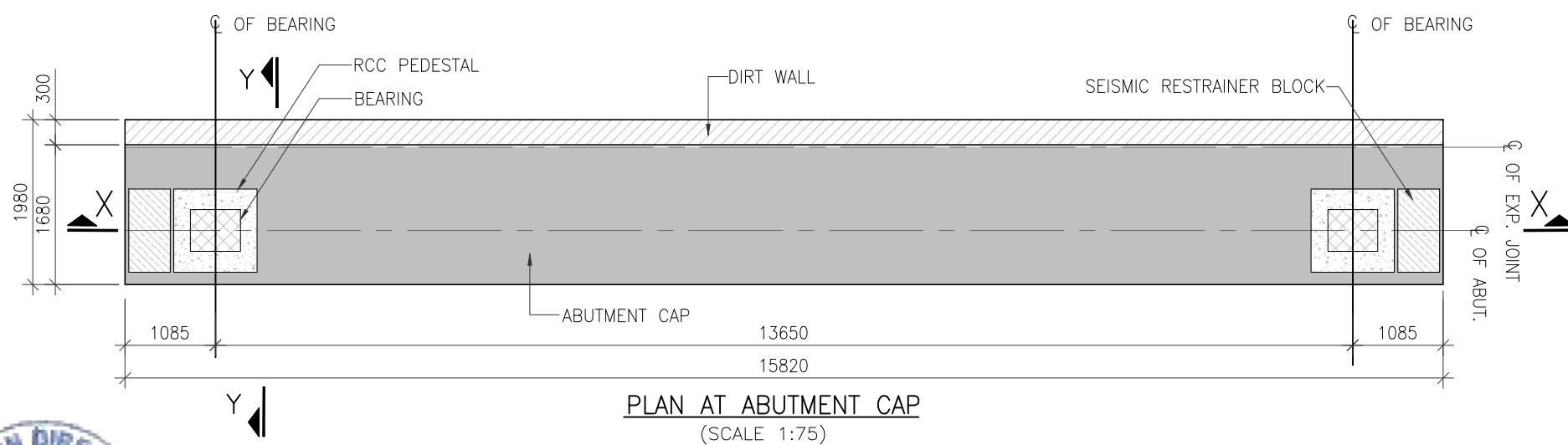
DRG. NO.STR/DWG/MJB/REINF./ABUT.-A1/202	DATE : SEPTEMBER. 2022	REV: R0
DESIGNED GS	DRAWN SP	CHECKED AS
APPROVED SM		



SECTION A-A
(SCALE 1:100)



SECTION X-X
(SCALE 1:75)



PLAN AT ABUTMENT CAP
(SCALE 1:75)

ABUTMENT LEVELS

LOCATION	FINISHED ROAD LEVEL	ABUTMENT CAP TOP LEVEL	ABUTMENT CAP BOTTOM LEVEL	GROUND LEVEL	FOUNDATION TOP LEVEL	FOUNDATION BOTTOM LEVEL
A2	1007.112	1004.897	1004.297	1004.112	1003.212	1002.012

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- DIMENSIONS ARE NOT TO BE SCALED, ONLY WRITTEN DIMENSIONS SHALL BE FOLLOWED.
- THE CARRIAGEWAY OF PROPOSED BRIDGE IS DESIGNED AS PER IRC : 6-2017.
- THE CARRIAGEWAY OF PROPOSED BRIDGE IS DESIGNED FOR 3 LANE OF CLASS A, 1 LANE OF 70R + 1 LANE CLASS A AND SV LOADING WHICHEVER GOVERNS AS PER IRC : 6-2017.
- BACK FILL MATERIALS SHALL CONSISTS OF SELECTED EARTH CONFORMING TO APPENDIX : 6 OF IRC : 78-2014.
- ALL LEVELS SHOWN HERE VERIFIED WITH APPROVED PLAN AND PROFILE DRAWING BY EXECUTING AGENCY/CONTRACTOR BEFORE START OF WORK.
- IN CASE OF POCKET OF SOFT UNSUITABLE STRATA IS FOUND BELOW FOUNDING LEVEL, SAME SHALL BE REPLACED WITH SUITABLE GRANULAR MATERIALS.
- FOR DETAILS OF APPROACHES, DRAINAGE SYSTEM, SLOPE PROTECTION, ETC, REFER SEPARATE DRAWINGS.
- 100mm DIA PVC PIPE SHALL BE PROVIDED AS PER WEEP HOLES @ 1000 C/C BOTHWAYS. WEEP HOLES SHALL BE PROVIDED AT A SLOPE OF 1:20.
- FILTER MEDIA BEHIND RETAINING WALL SHALL BE PROVIDED AS PER APPENDIX : 6 OF IRC : 78-2014.
- THE DESIGN AND DETAILING IS CARRIED OUT WITH FOLLOWING ASSUMPTIONS AS PER CLAUSE 4.2 OF IRC : 112-2011.
- MODERATE EXPOSURE CONDITION IS CONSIDERED FOR DESIGN
- THE BRIDGE SITE IS LOCATED IN SEISMIC ZONE-IV.
- THE STRUCTURE IS DESIGNED FOR 100 YEARS OF A LIFE TIME.
- THE SATURATED DENSITY OF SOIL IS 20 kN/m³ AND THE SUBMERGED DENSITY OF SOIL IS 10 kN/m³.
- BACK FILLING BEHIND ABUTMENTS SHALL CONSIST OF SELECTED EARTH CONFORMING TO APPENDIX :6 OF IRC:78-2000 HAVING PROPERTIES C=0 , $\phi=33^\circ$.

GRADES, MATERIAL TYPES & ETC.:-

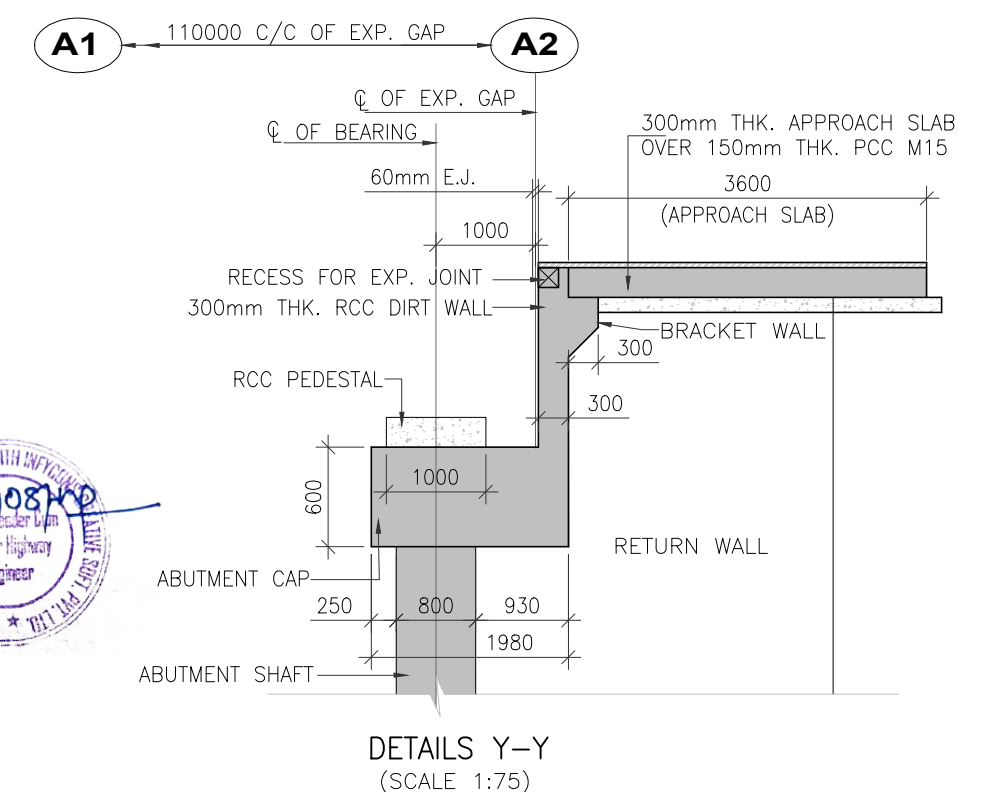
- RCC CRASH BARRIERM40
- SUBSTRUCTURE / FOUNDATION.....M35
- APPROACH SLABM30
- ABUTMENT CAPM35
- PEDESTAL.....M40
- PCCM15
- SEISMIC ZONE.....(IV)
- B.C WEARING COAT.....50MM THK.
- STEEL/HYSD BARS (CONFIRM TO IS:1786).....Fe500D.

CLEAR COVER:-

- RCC CRASH BARRIER50mm
- FOUNDATION75mm
- RCC ABUTMENT SHAFT (EARTH FACE)75mm
- RCC ABUTMENT SHAFT (NON-EARTH FACE).....40mm
- ABUTMENT CAP.....50mm
- DIRT WALL50mm
- APPROACH SLAB50mm

SPECIAL NOTES:

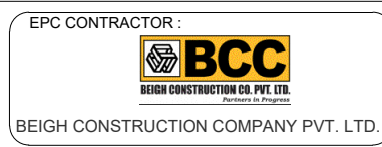
- "SAFE BEARING PRESSURE CONSIDERED IN DESIGN IS 65 T/m² AND THE SAME SHALL BE VERIFIED AT SITE BEFORE EXECUTION OF WORK."
- "PLATE LOAD TEST SHALL BE DONE AT SITE."



DETAILS Y-Y
(SCALE 1:75)

PLAN OF FOUNDATION
(SCALE 1:75)

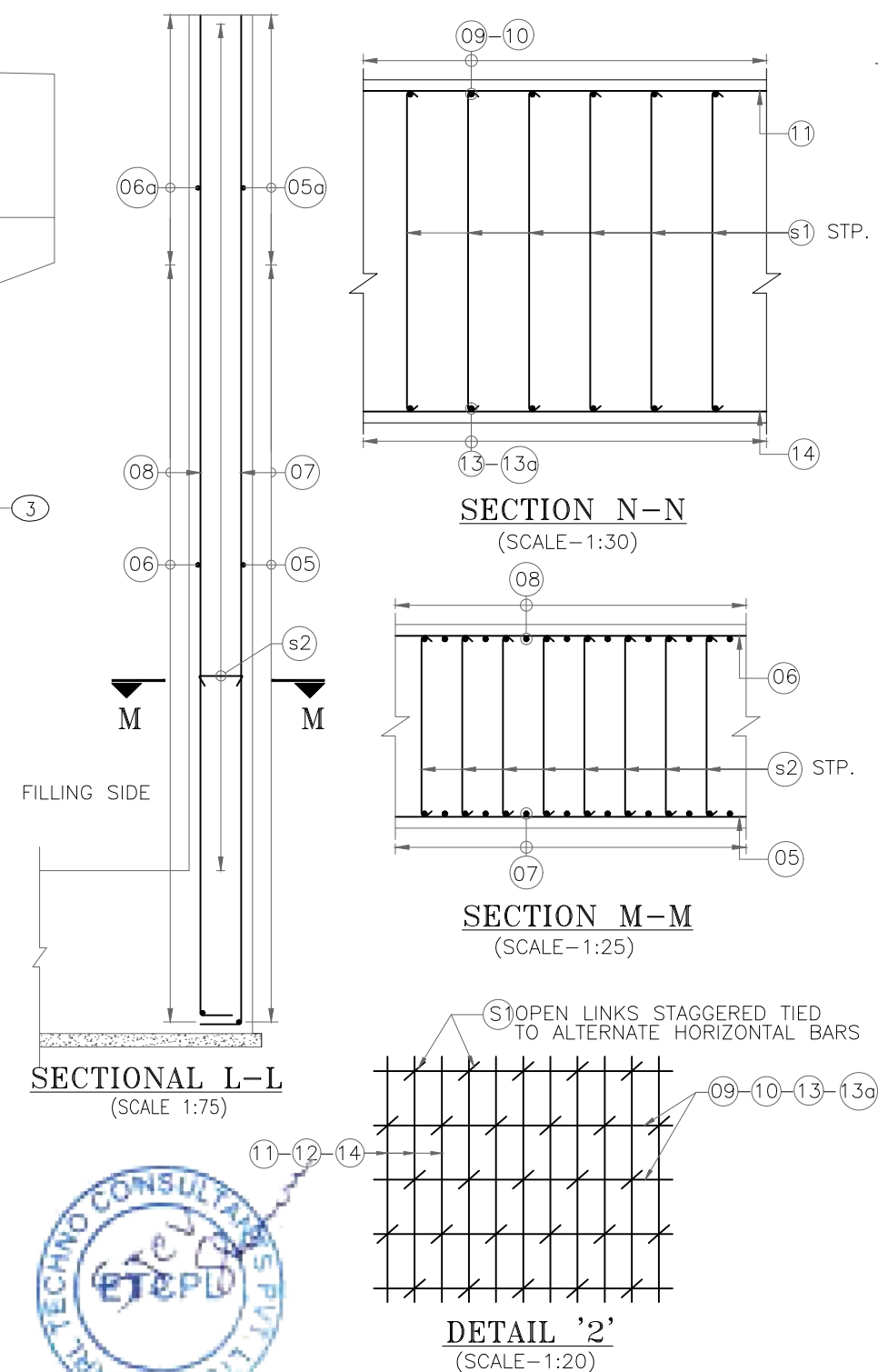
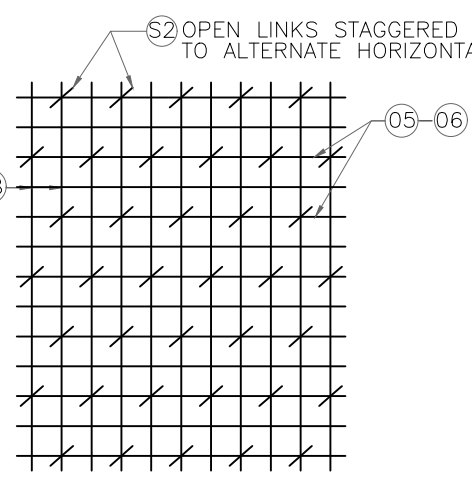
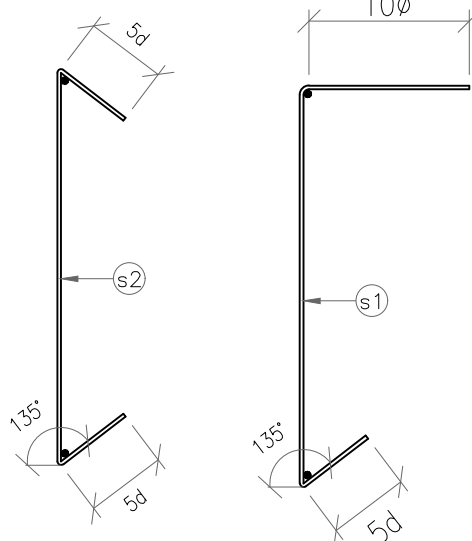
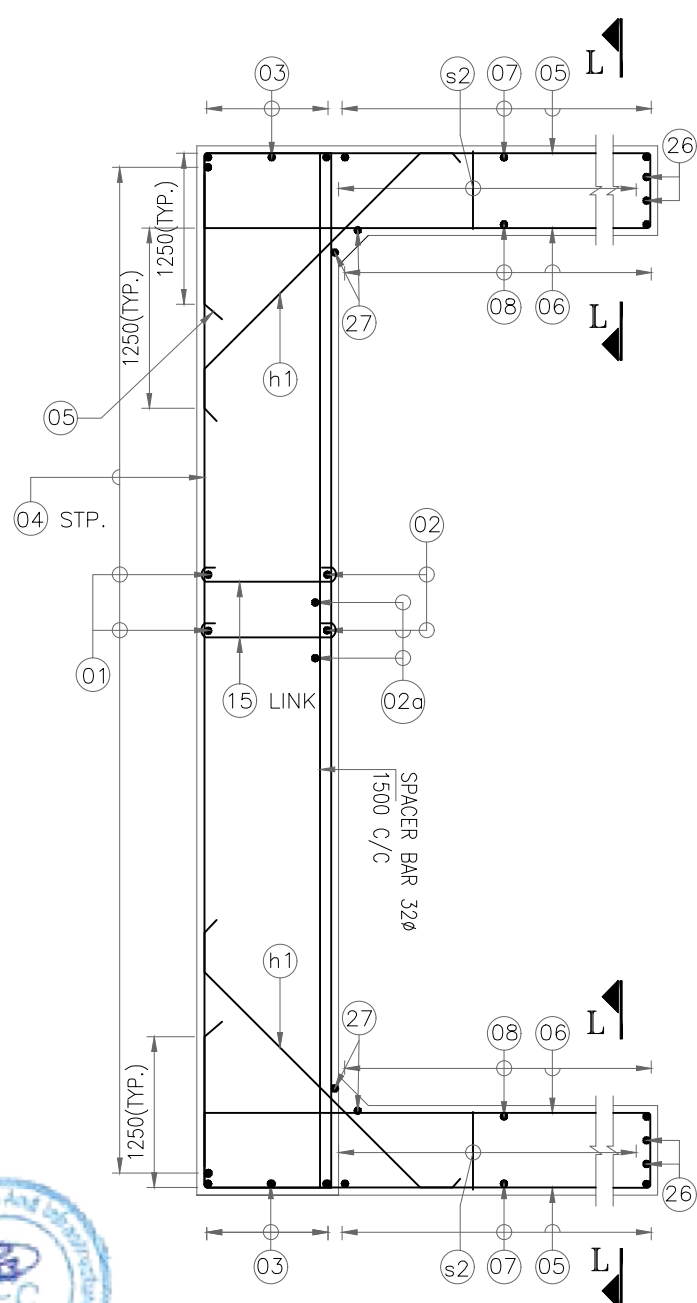
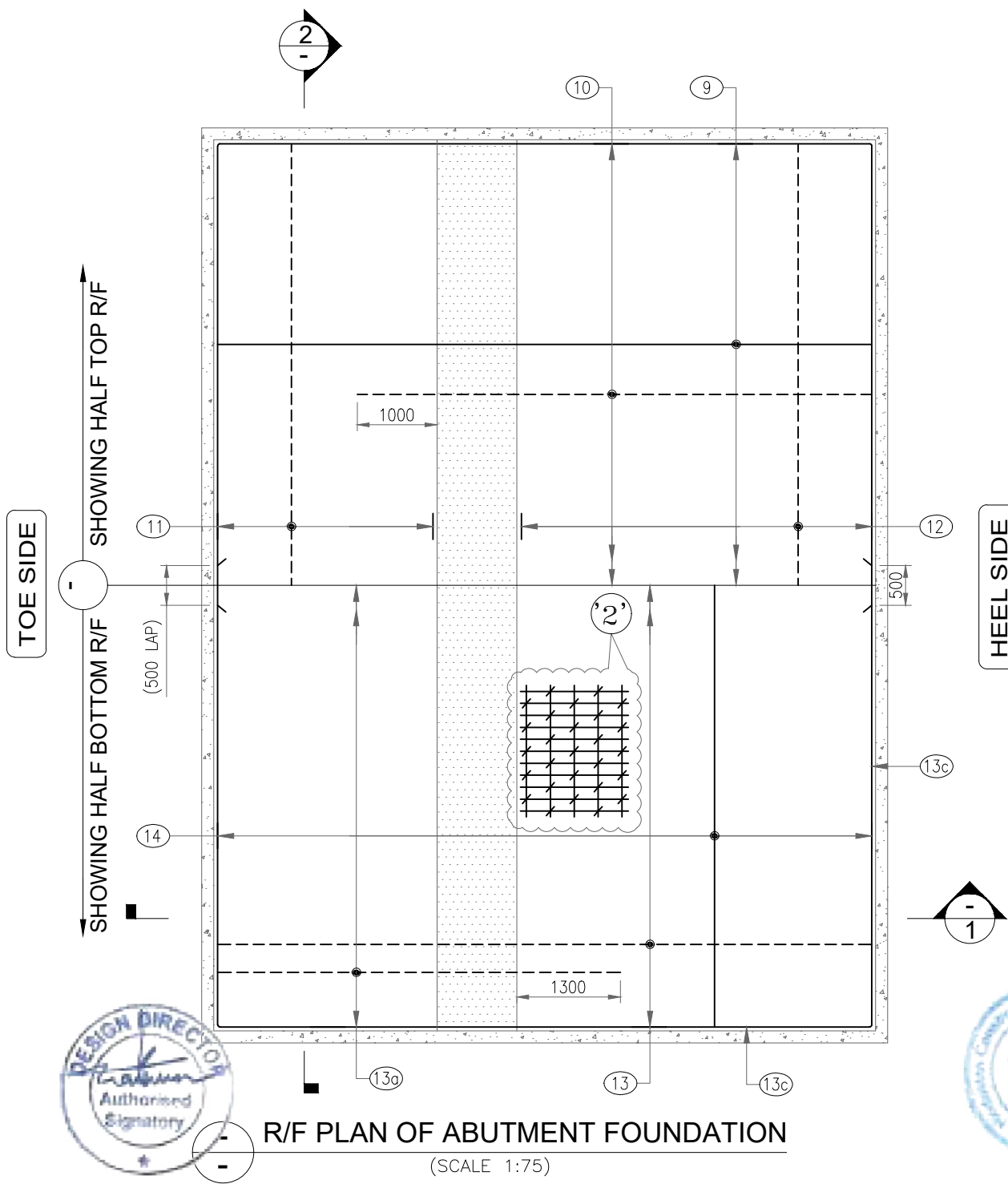
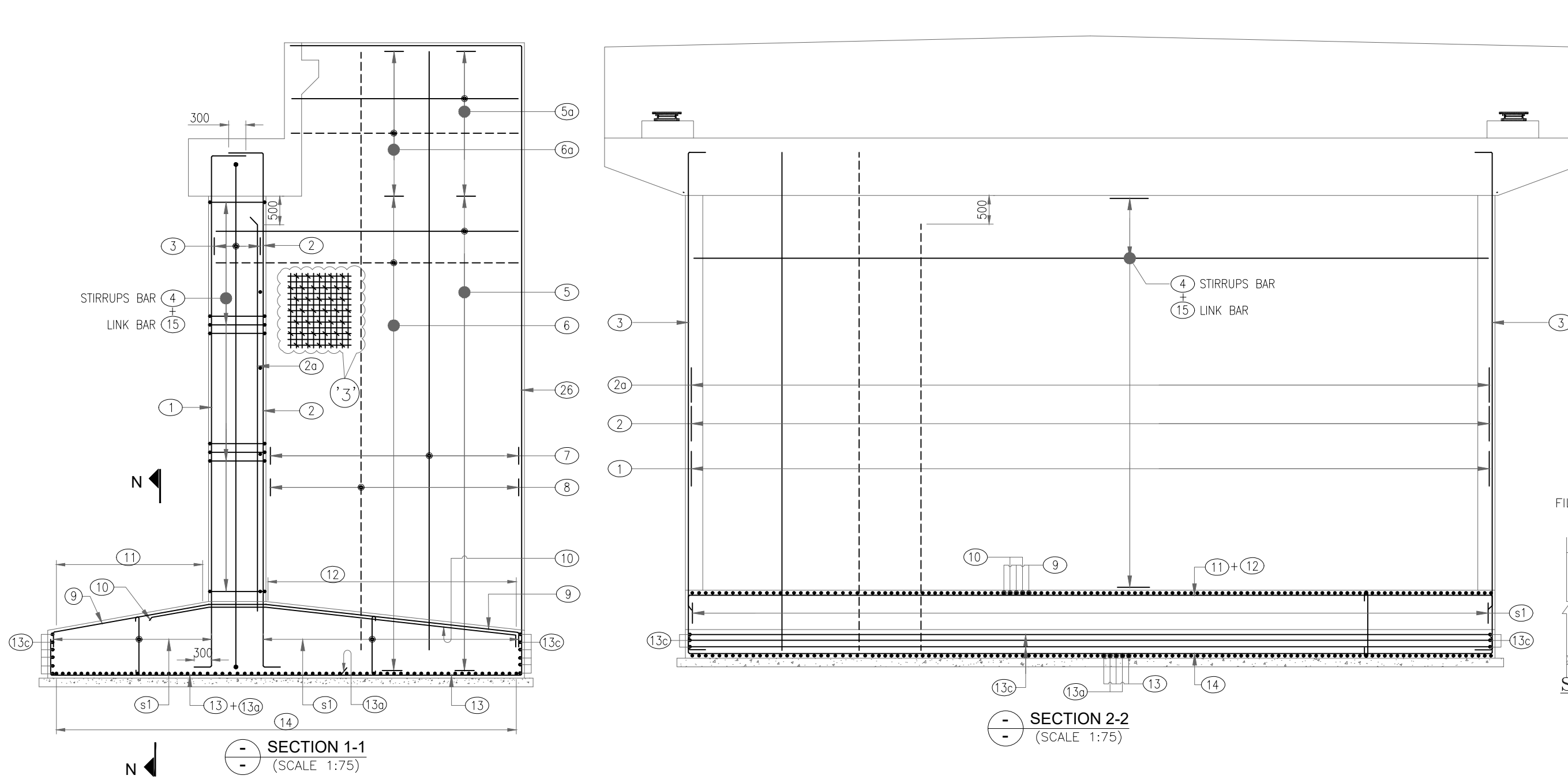
REV	DATE	DESCRIPTION	A	B	C	D
R0	SEP-2022	SUBMISSION OF MJB				



PROJECT :
CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT KM 83RD OF LENGTH 495M INCLUDING APPROACH ROAD OF 664M AND MAJOR BRIDGE OF 110M FROM KM 82+675 TO KM 82+925 ON NH-244 IN UT OF JAMMU AND KASHMIR ON EPC MODE.

DRAWING TITLE :
DIMENSION DETAILS DRAWING OF ABUTMENT (A2) FOR MJB AT CH. 0+689

DRG. NO. STR/DWG/MJB/DIM./ABUT.-A2/203			
DATE : SEPTEMBER. 2022		REV: R0	
DESIGNED GS	DRAWN SP	CHECKED AS	APPROVED SM



NOTE'S:-

- ALL DIMENSIONS ARE IN MILLIMETERS, LEVELS ARE IN METERS AND CHAINAGES ARE IN KILOMETERS UNLESS OTHERWISE MENTIONED.
- DIMENSIONS ARE NOT TO BE SCALED, ONLY WRITTEN DIMENSIONS SHALL BE FOLLOWED.
- THE CARRIAGEWAY OF PROPOSED BRIDGE IS DESIGNED AS PER IRC : 6-2017.
- ADEQUATE LINKS SPACER BARS ETC SHALL BE PROVIDED FOR PROPER POSITIONING OF REINFORCEMENT.
- LAP JOINT SHALL BE PROVIDED IN STAGGERED MANNER & NOT MORE THAN 50% BAR SHALL BE LAPPED AT ANY SECTION.
- BOND LENGTH SHALL BE 50 TIMES DIA OF BAR & LAP LENGTH SHALL BE 73 TIMES DIA OF BAR.
- ABUT. CAP TOP LEVEL AND PEDESTAL HEIGHT MAY BE VARIES AFTER DESIGN OF BEARING SIZE.

GRADES, MATERIAL TYPES & ETC:-

a) RCC CRASH BARRIER	M40
b) SUBSTRUCTURE / FOUNDATION.....	M35
c) APPROACH SLAB	M30
d) ABUTMENT CAP	M35
e) PEDESTAL.....	M40
f) PCC	M15
g) SEISMIC ZONE.....	(IV)
h) B.C WEARING COAT.....	50MM THK.
i) STEEL/HYSD BARS (CONFIRM TO IS:1786).....	Fe500D.

CLEAR COVER:-

a) RCC CRASH BARRIER	50mm
b) FOUNDATION	75mm
c) RCC ABUTMENT SHAFT (EARTH FACE)	75mm
d) RCC ABUTMENT SHAFT (NON-EARTH FACE).....	40mm
e) ABUTMENT CAP.....	50mm
f) DIRT WALL	50mm
g) APPROACH SLAB	50mm

SPECIAL NOTES:-

- "SAFE BEARING PRESSURE CONSIDERED IN DESIGN IS 6.5 T/m² AND THE SAME SHALL BE VERIFIED AT SITE BEFORE EXECUTION OF WORK."
- "PLATE LOAD TEST SHALL BE DONE AT SITE."

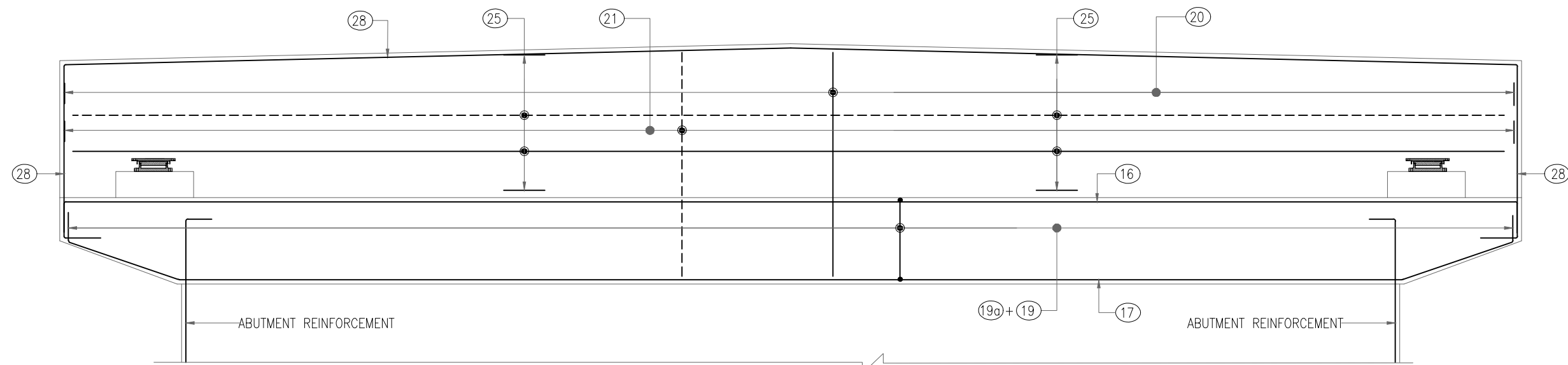
REINFORCEMENT SCHEDULE FOR ABUTMENT-A2

BAR MKD.	SHAPE (NOT TO SCALE)	DIA. (mm)	TOTAL NOS. / SPACING (mm)
01		16	160 C/C
02		20	160 C/C
02a	BAR MARK NOT USED		
03		12	150 C/C
04		12	150 C/C
05		12	150 C/C
05a		12	150 C/C
06		20	150 C/C
06a		16	150 C/C
07		12	150 C/C
08		20	150 C/C
09		20	100 C/C
10		BAR MARK NOT USED	
11		16	200 C/C
12		16	200 C/C
13		25	100 C/C
13a		16	100 C/C
13b	BAR MARK NOT USED		
13c		16	100 C/C
14		20	200 C/C
15		10	200 C/C VERTICALLY
26		12	2 x 5 NOS.
h1		10	100 C/C
s1		10	OPEN LINK 200 C/C
s2		10	OPEN LINK 200 C/C
27		12	2 x 2 NOS.

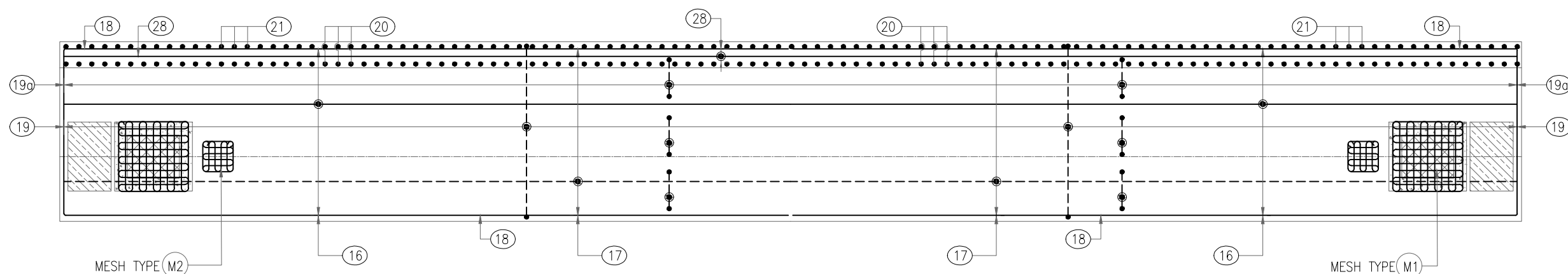
LEGEND:

TOP REIN. _____

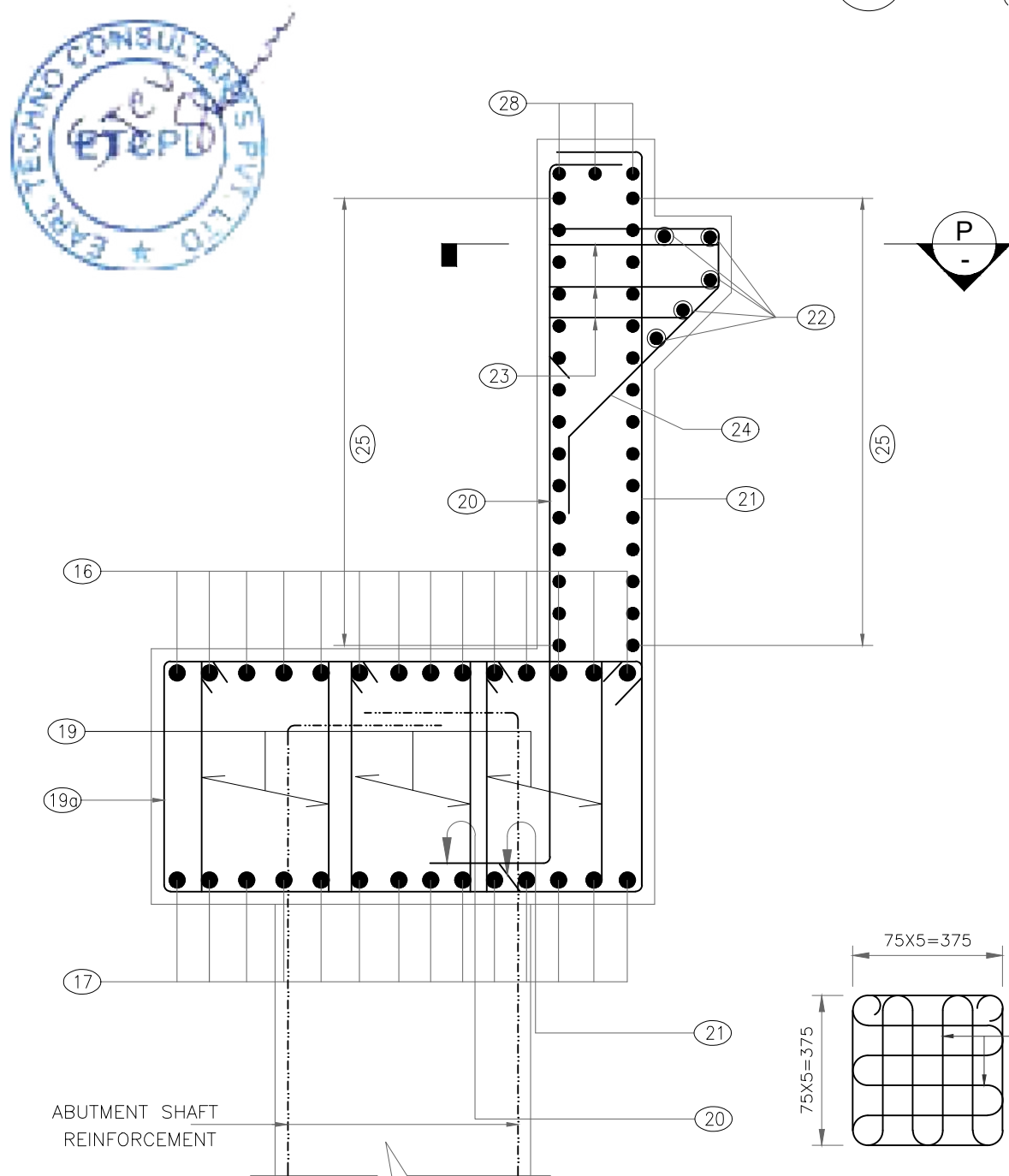
BOTTOM REIN. - - - - -



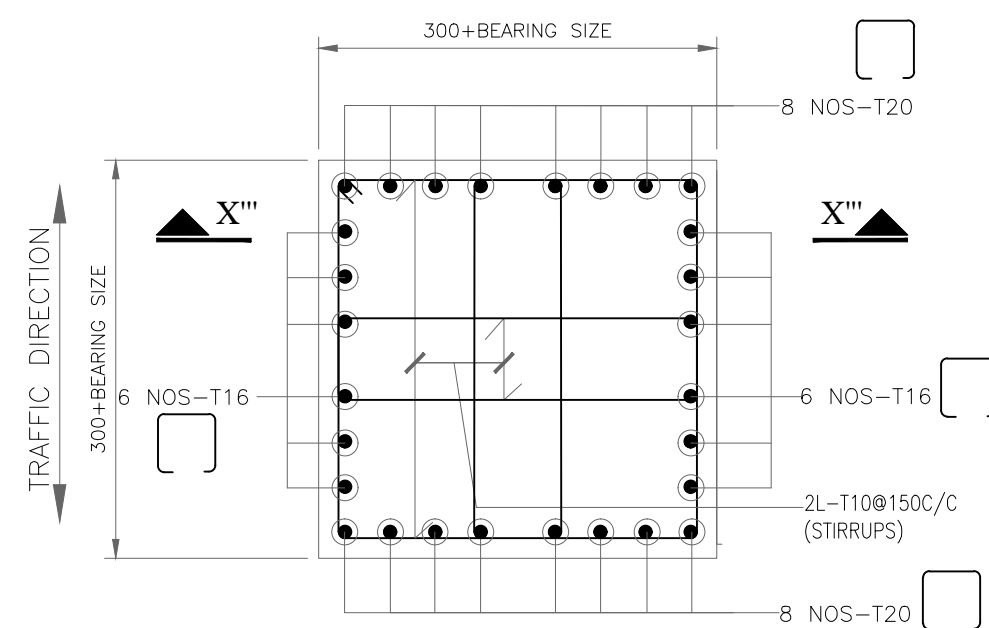
SECTION 2-2
(SCALE 1:50)



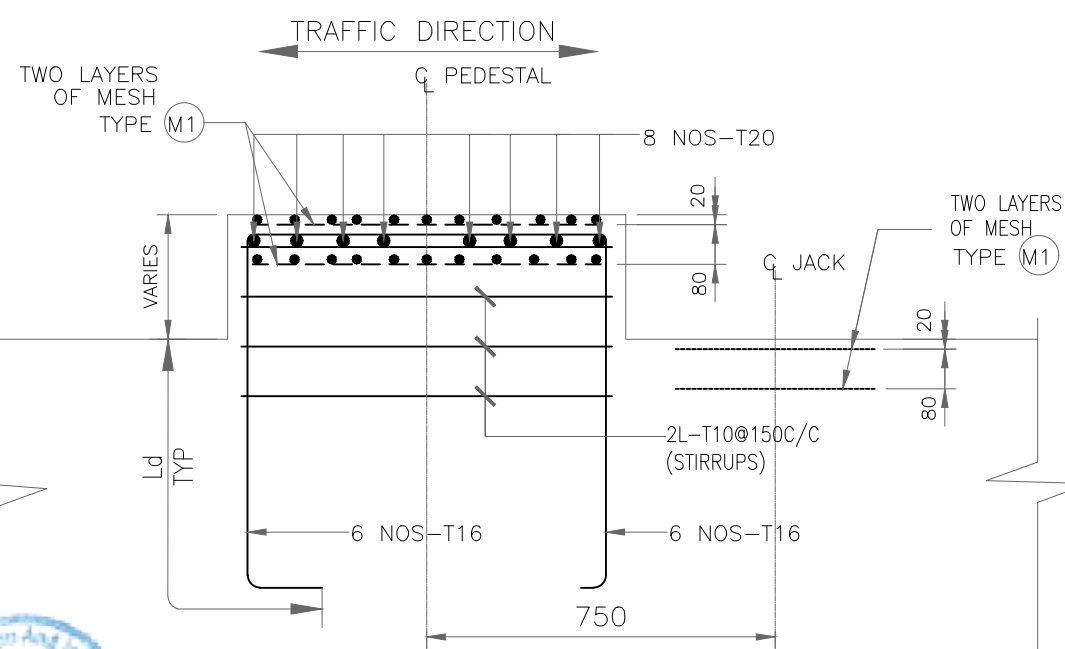
PLAN AT ABUTMENT CAP
(SCALE 1:75)



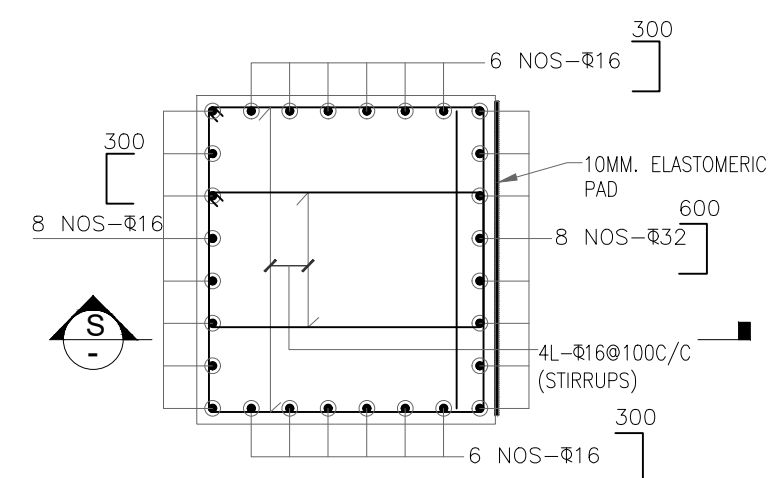
R/F DETAILS OF ABUT. CAP
(SCALE 1:40)



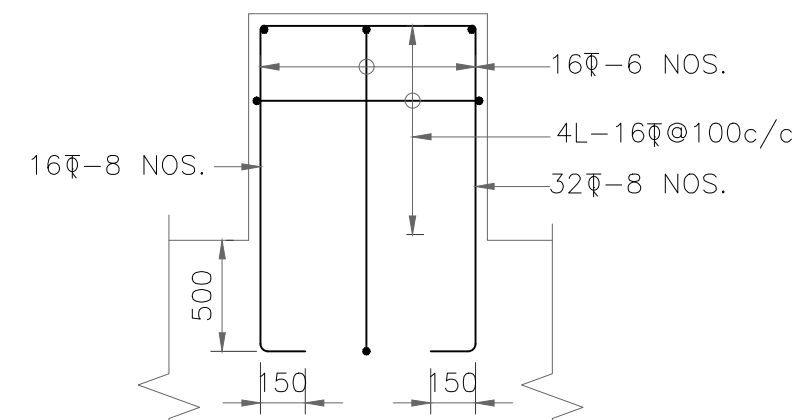
PLAN OF BEARING PEDESTAL
(SCALE 1:15)



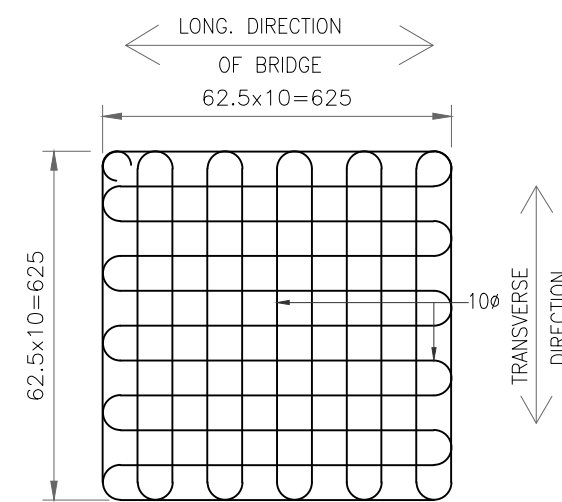
SECTION AT X'''- X'''
(SCALE 1:15)



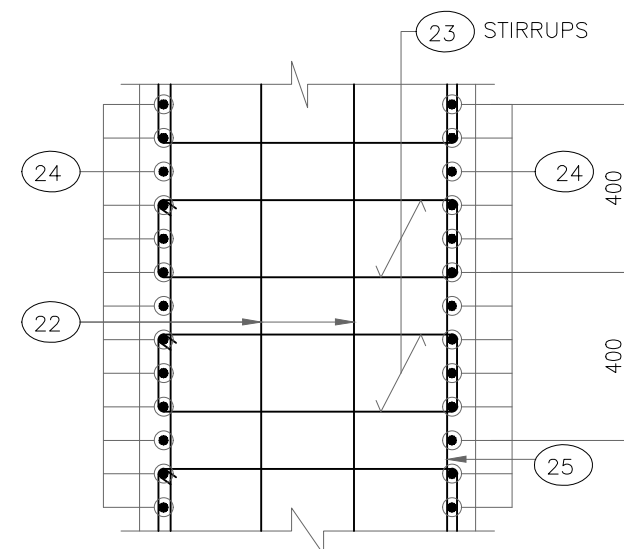
R/F DETAILS OF SEISMIC BLOCK
(SCALE 1:25)



SECTION S-S
(SCALE 1:30)



TYPE-M1
(AT BEARINGS LOCATIONS)
(IN TWO LAYERS)
(SCALE 1:15)



SECTION P-P
(SCALE 1:25)

REINF.SCHEDULE FOR ABUTMENT-A2

BAR MKD.	SHAPE (NOT TO SCALE)	DIA. (mm)	TOTAL NOS. / SPACING (mm)
16		16	150 C/C
17		16	150 C/C
18	NOT IN USE		
19		12	200 C/C
19a		12	200 C/C
20		12	150 C/C
21		12	150 C/C
22		10	6 NOS.
23		10	400 C/C 2 LEGGED 3 NOS.(HORI.)
24		12	150 C/C
25		10	150 C/C
28		16	3 NOS.

NOTE'S:-

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- DIMENSIONS ARE NOT TO BE SCALED, ONLY WRITTEN DIMENSIONS SHALL BE FOLLOWED.
- THE CARRIAGEWAY OF PROPOSED BRIDGE IS DESIGNED AS PER IRC : 6-2017.
- ADEQUATE LINKS SPACER BARS ETC SHALL BE PROVIDED FOR PROPER POSITIONING OF REINFORCEMENT.
- LAP JOINT SHALL BE PROVIDED IN STAGGERED MANNER & NOT MORE THAN 50% BAR SHALL BE LAPPED AT ANY SECTION.
- BOND LENGTH SHALL BE 50 TIMES DIA OF BAR & LAP LENGTH SHALL BE 73 TIMES DIA OF BAR.
- ABUT. CAP TOP LEVEL AND PEDESTAL HEIGHT MAY BE VARIES AFTER DESIGN OF BEARING SIZE.

GRADES, MATERIAL TYPES & ETC.:-

- RCC CRASH BARRIERM40
- SUBSTRUCTURE / FOUNDATION.....M35
- APPROACH SLABM30
- ABUTMENT CAPM35
- PEDESTAL.....M40
- PCCM15
- SEISMIC ZONE.....(IV)
- B.C WEARING COAT.....50MM THK.
- STEEL/HYSD BARS (CONFIRM TO IS:1786).....Fe500D.

CLEAR COVER:-

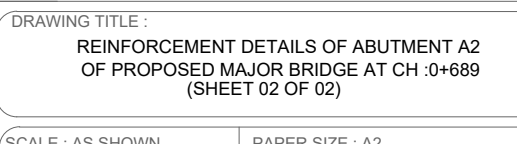
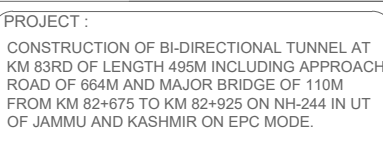
- RCC CRASH BARRIER50mm
- FOUNDATION75mm
- RCC ABUTMENT SHAFT (EARTH FACE)75mm
- RCC ABUTMENT SHAFT (NON-EARTH FACE).....40mm
- ABUTMENT CAP.....50mm
- DIRT WALL50mm
- APPROACH SLAB50mm

SPECIAL NOTES:

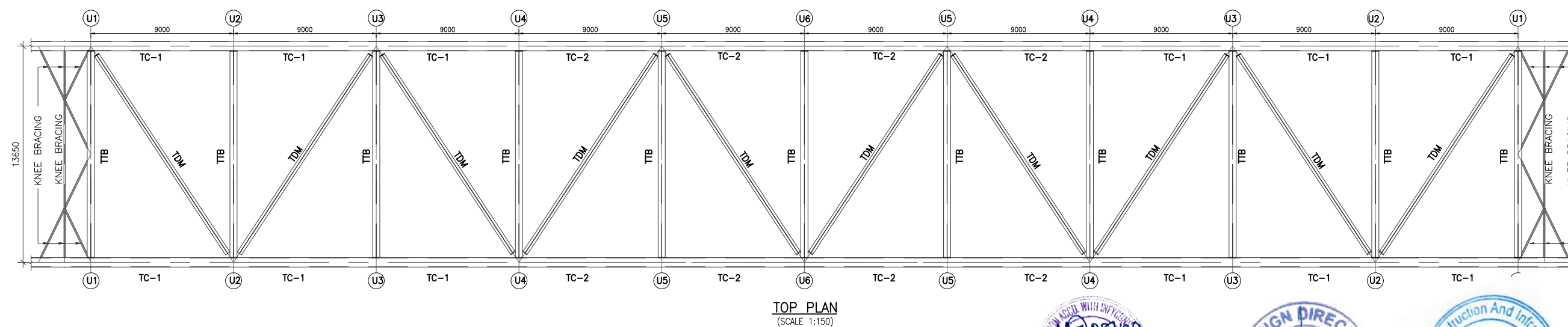
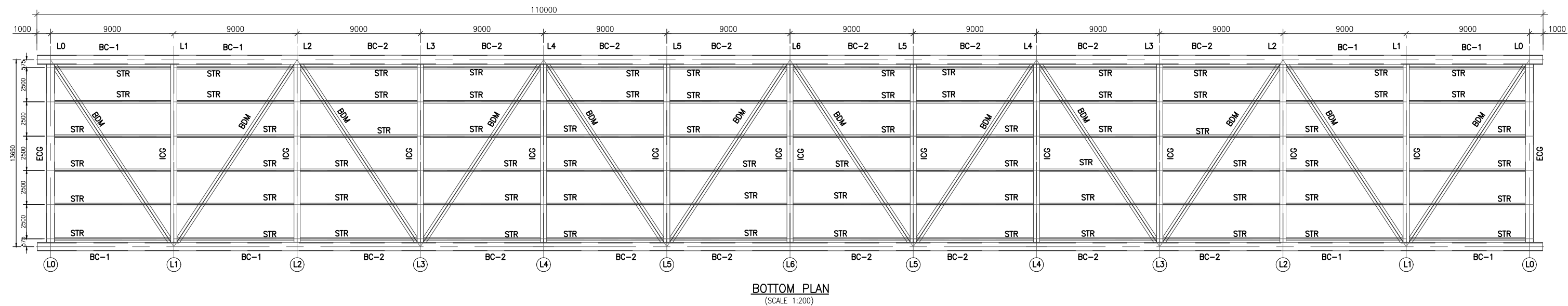
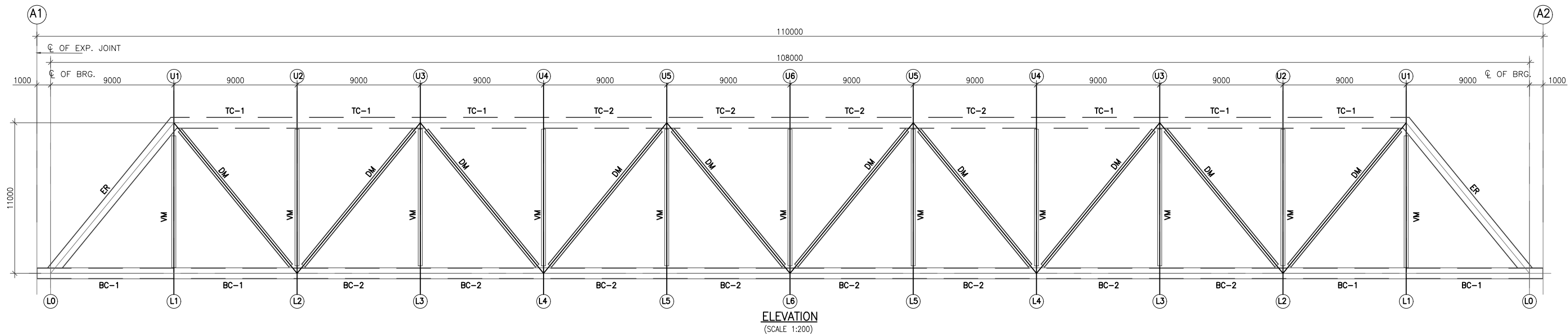
- "SAFE BEARING PRESSURE CONSIDERED IN DESIGN IS 70 T/m² AND THE SAME SHALL BE VERIFIED AT SITE BEFORE EXECUTION OF WORK."
- "PLATE LOAD TEST SHALL BE DONE AT SITE."



REV	DATE	DESCRIPTION	A	B	C	D
R0	SEP-2022	SUBMISSION OF MJB				



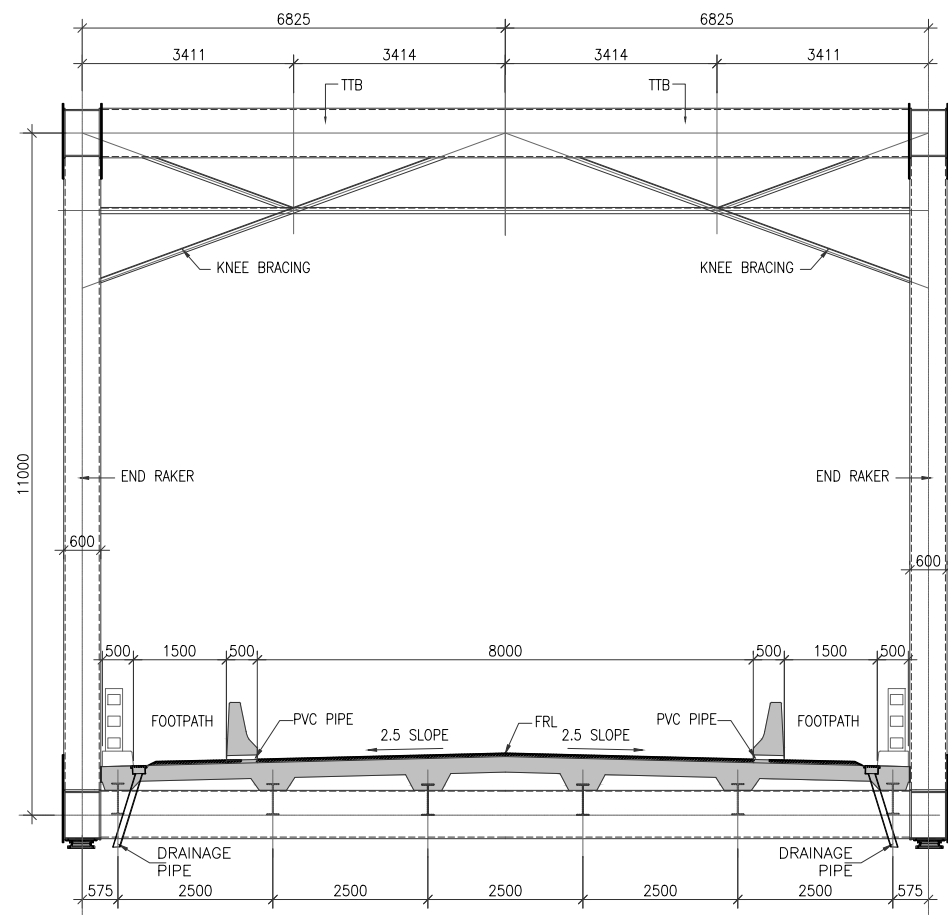
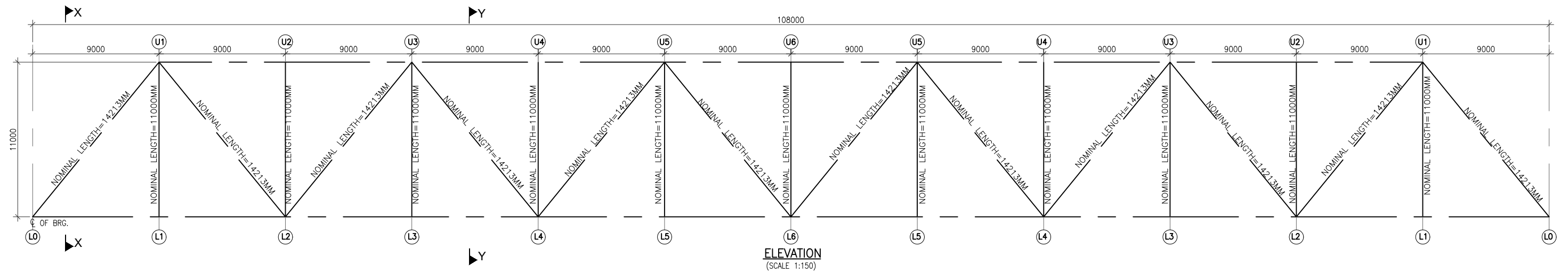
DRG. NO.STR/DWG/MJB/REINF./ABUT.-A2/204	REV: R0
DATE : SEPTEMBER, 2022	DESIGNED GS
CHECKED AS	DRAWN SP
APPROVED SM	PAPER SIZE : A2



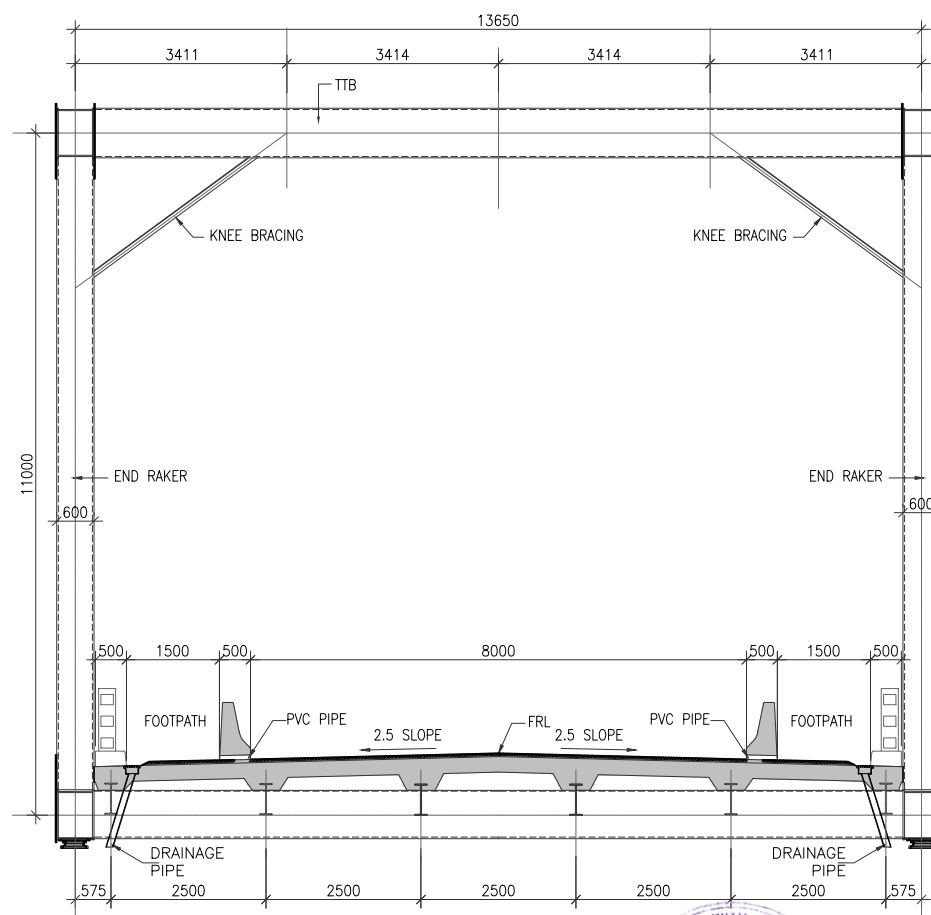
LIST OF ABBREVIATIONS--:

S.No.		
1	TC-1,2	TOP CHORD
2	BC-1,2	BOTTOM CHORD
3	ER	END RAKER
4	TDM	TOP DIAGONAL MEMBER
5	BDM	BOTTOM DIAGONAL MEMBER
6	DM	DIAGONAL BRACING
7	STR	STRINGERS
8	ICG	INNER CROSS GIRDER
9	ECG	END CROSS GIRDER
10	VM	VERTICAL MEMBER
11	TTB	TOP TIE BRACING





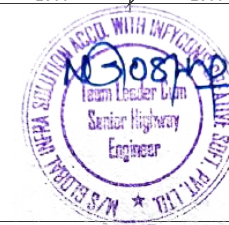
SECTION X-X
(SCALE 1:85)



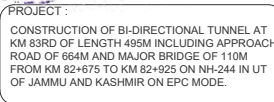
SECTION Y-Y
(SCALE 1:85)

NOTES:-

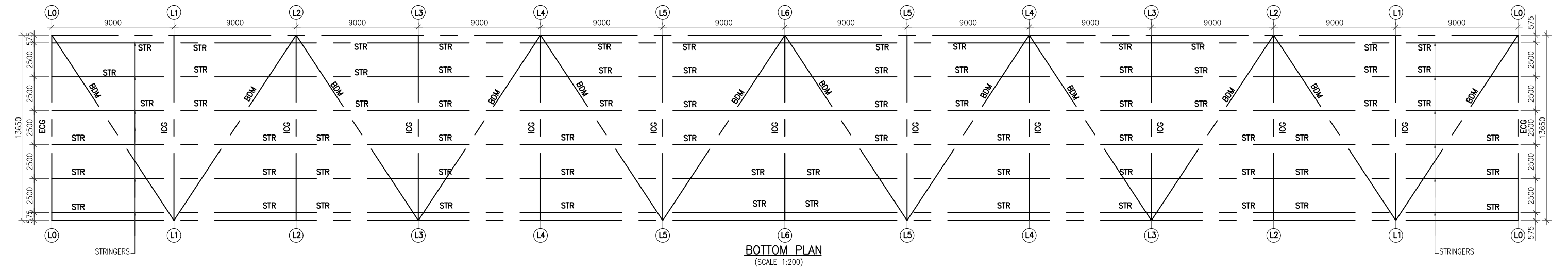
1. ALL DIMENSIONS ARE IN MM AND LEVELS ARE IN METRES UNLESS OTHERWISE MENTIONED.
2. DIMENSIONS ARE NOT TO BE SCALED. ONLY WRITTEN DIMENSIONS ARE TO
3. DURING WELDING POSITION OF MEMBERS AND ASEMBLY SHOULD BE SUCH THAT OVERHEAD WELDING SHALL BE AVOIDED/MINIMISED.
4. ALL THE MEMBERS MEETING AT A JOINTS SHOULD BE WELDED AROUND.
5. ALL WELDING ELECTRODES FOR 'SMAW' PROCESS SHALL CONFORM TO AWS A 505.
6. ALL BARE ELECTRODES FOR 'GMAW' PROCESS SHALL CONFORM TO AWS A SUBMERGED ARC WELDING (SAW) SHALL BE AS PER AWS A 5.23.
7. ALL WELDING TO BE DONE IN CONFORMITY WITH IS:9595 & OR AWS D 1.1/D 1.5 & OTHER RELEVANT SPECIFICATION.
8. BREAKUP OF DIMENSIONS SHOULD TALLY WITH OVERALL DIMENSION FOR ALL THE MEMBERS.
9. STRIP SEAL EXPANSION JOINTS SHALL BE PROVIDED ON EXPANSION END.
10. BEARINGS SHALL BE POT/PTE BEARINGS.
11. GRADE OF DECK SLAB IS M-40.
12. THE DESIGN IS IN ACCORDANCE WITH IRC:5-1998, IRC:6-2014, IRC:112-2011 IRC:22-2008 &IRC:24-2010.
13. HIGH STRENGTH FRICTION GRIP BOLT (HSFG) OF GRADE (10.9) MAY BE USED IN CONFORMITY WITH IS:4000- 1992. SLIP FACTOR SHOULD BE TAKEN 0.35.
14. FOR LIFTING OF SPAN IN FUTER, JACKS WELL BE LOCTED UNDER END CROSS GRIDERS AT THE LOCATION SHOWN.
15. WHEN THE SPAN IS LIFTED ON JACKS LIVE LOAD WILL NOT BE FLOWED OVER THE BRIDGE, JACK REQUIRED FOR LIFTING OF SPAN SHALL HAVE MINIMUM CAPACITY OF 250 TON
16. DO NOT PROVIDE ANY PAINT SUCH AS RED OXIDE, PRIMER OR ANY OTHER COATING OVER HSFG BOLT CONNECTION AREA



REV	DATE	DESCRIPTION	A	B	C	D
RD	SEP-2022	SUBMISSION OF MJB				



DRG. NOSTR/DWG/MJB/SUPER-STRUCTURE-301			
DATE : SEPTEMBER, 2022		REV: R0	
DESIGNED GS	DRAWN SP	CHECKED AS	APPROVED SM



S.NO.	01.	02.	03.	04.	05.	06.	07.	08.
MEMBER MRK.	BOTTOM CHORD (BC-1) L0-L1, L1-L2	BOTTOM CHORD (BC-2) L2-L3, L3-L4, L4-L5, L5-L6	TOP CHORD (TC-1) U1-U2, L2-L3, U3-U4	TOP CHORD (TC-2) U4-U5, U5-U6	VERTICAL MEMBER (VM)	TOP DIAGONAL MEMBER (TDM)	STRINGERS (STR)	INNER CROSS GIRDER (ICG)
SHAPE								
WEB	① 20	④ 22	⑦ 20	⑩ 20	⑬ 20	⑯ 18	⑲ 28	②② 28
TOP FLANGE	② 18	⑤ 20	⑧ 18	⑪ 18	⑭ 12	⑰ 12	⑳ 20	②③ 28
BOTTOM FLANGE	③ 12	⑥ 12	⑨ 12	⑫ 12	⑮ 12	⑱ 12	㉑ 20	②④ 28



NOTES:-

- ALL DIMENSIONS ARE IN MM AND LEVELS ARE IN METRES UNLESS OTHERWISE MENTIONED.
- DIMENSIONS ARE NOT TO BE SCALED. ONLY WRITTEN DIMENSIONS ARE TO BE FOLLOWED.
- FOR ERECTION / LAUNCHING SCHEME SEE SEPARATE DRAWING.
- ALL BUTT WELDED ARE TO BE EXAMINED RADIOGRAPHICALLY OR ANY EQUALLY EFFECTIVE NON DESTRUCTIVE TEST METHOD.
- ALL WELDS TO BE MADE BY APPROVED WELDING PROCEDURES.
- FABRICATION SHALL BE DONE AS PER IRC: 24-2010.
- FILLET WELDS ARE OF LEG SIZE 6MM MINIMUM AND AS PER IS:CODE 9595-1996.
- ALL OBTUSE ANGLE CUTTING SHOULD BE DONE IN PROPER SHAPE & WITHOUT DAMAGE.
- OVERHANGING MEMBER TO BE SUITABLY SUPPORTED TO AVOID DISTORTION DURING TRANSPORTATION.
- ALL WELDS SHALL BE IN ONE GO (CONTINUOUS) UNLESS SPECIFIED OTHERWISE.
- FOR DESIGN OF BRIDGE SEISMIC ZONE-IV IS CONSIDERED.
- ALL STRUCTURAL STEEL PLT SHALL BE E350 CONFORMING TO IS 2062-2006.
- FOR FABRICATION, WORKMANSHIP, INSPECTION & TESTING PROTECTION AGAINST CORROSION ETC, RELEVANT PROVISION OF IRC 24-2010 SHALL BE FOLLOWED.
- CONSTRUCTION STAGE SCHEME SHALL BE PROVIDED BY CONTRACTOR AND CONSTRUCTION SHALL BE VERIFIED BY DDC BEFORE FABRICATION.
- CONGESTION FACTOR IS NOT CONSIDERED.

WELDING NOTES:-

- WELDING ELECTRODES SHALL CONFIRM IS 814 OR IS 1395
- ALL WELDING ELECTRODES OR "SMW" PROCESS SHALL CONFORM A 5.5 FOR HT & AWS A 5.1 FOR MS.
- METAL ARC WELDING SHALL CONFORM TO IS:9595 & 816/ AWS D 1.1/1.5.
- FOR SCHEME & SYMBOLS OF WELDING REFER IS:813-1986.
- ALL WELDING TO BE DONE IN CONFIRMITY WITH IS:9595 & OR AWS D 1.1/D1.5& OTHER RELEVANT SPECIFICATION.

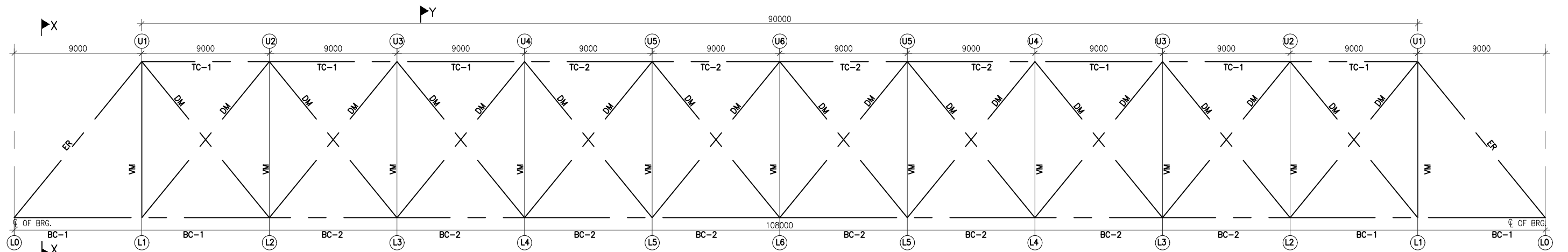
S.NO.	09.	10.	11.	12.	13.	14.
MEMBER MRK.	END CROSS GIRDER (ECG)	END RAKER (ER)	TOP TIE BRACING (TTB)	BOTTOM DIAGONAL MEMBER (BDM)	DIAGONAL MEMBER (DM)	KNEE BRACING
SHAPE						
WEB	②⑤ 32	②⑧ 20	③① 18	③④ 16	③⑦ 22	
TOP FLANGE	②⑥ 32	②⑨ 22	③② 12	③⑤ 12	③⑧ 22	
BOTTOM FLANGE	②⑦ 32	③⑩ 22	③③ 12	③⑥ 12	③⑨ 22	

LIST OF ABBREVIATIONS:-

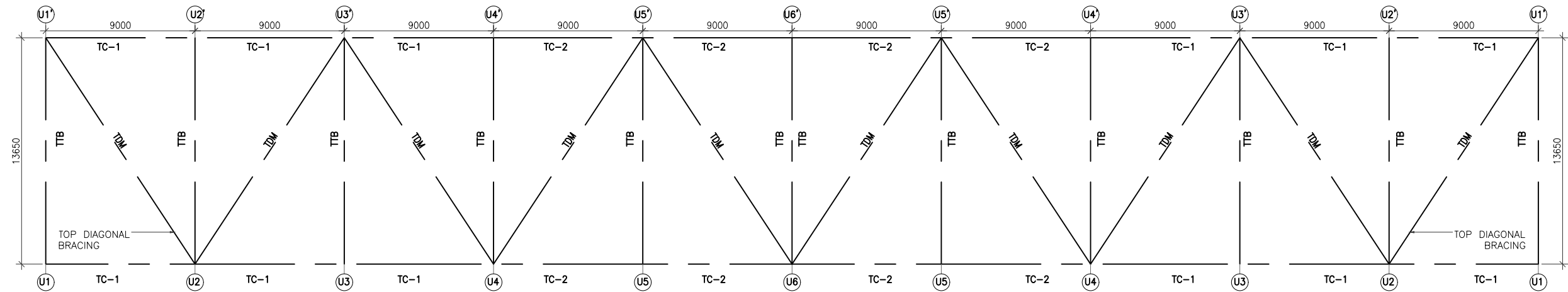
S.No.		
1	TC-1,2	TOP CHORD
2	BC-1,2	BOTTOM CHORD
3	ER	END RAKER
4	TDM	TOP DIAGONAL MEMBER
5	BDM	BOTTOM DIAGONAL MEMBER
6	DM	DIAGONAL BRACING
7	STR	STRINGERS
8	ICG	INNER CROSS GIRDER
9	ECG	END CROSS GIRDER
10	VM	VERTICAL MEMBER
11	TTB	TOP TIE BRACING



							CLIENT:  NATIONAL HIGHWAYS AND INFRASTRUCTURE DEVELOPMENT CORPORATION LTD.		AUTHORITY: 		EPC CONTRACTOR:  BEIGH CONSTRUCTION COMPANY PVT. LTD.		PROOF CONSULTANT:  HINDUSTAN CONSTRUCTION & INFRASTRUCTURE CONSULTANTS		SAFETY CONSULTANT:  EARL TECHNO CONSULTANTS PVT. LTD.		DESIGN CONSULTANT:  FUTURE BUILD COMPANY		PROJECT: CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT KM 83RD OF LENGTH 495M INCLUDING APPROACH ROAD OF 664M AND MAJOR BRIDGE OF 110M FROM KM 82+675 TO KM 82+925 ON NH-244 IN UT OF JAMMU AND KASHMIR ON EPC MODE.		DRAWING TITLE: GENERAL ARRANGEMENT DRAWING OF SUPER STRUCTURE FOR MJB AT CH -0+689 (SPAN 1x110.0M) (SHEET-03 OF 04)		DRG. NO.STR/DWG/MJB/SUPER-STRUCTURE-301			
R0		SEP-2022		SUBMISSION OF MJB															DATE : SEPTEMBER, 2022		REV: R0					
REV		DATE		DESCRIPTION			A		B		C		D						DESIGNED GS		DRAWN SP		CHECKED AS		APPROVED SM	



ELEVATION
(SCALE 1:125)



TOP PLAN
(SCALE 1:125)



REV	DATE	DESCRIPTION	A	B	C	D
01	SEP-2022	SUBMISSION OF MJB				



PROJECT :
CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT
KM 83RD OF LENGTH 495M INCLUDING APPROACH
ROAD OF 664M AND MAJOR BRIDGE OF 110M
FROM KM 82+675 TO KM 82+925 ON NH-244 IN UT
OF JAMMU AND KASHMIR ON EPC MODE.

DRAWING TITLE :
GENERAL ARRANGEMENT DRAWING OF
SUPER STRUCTURE FOR MJB AT CH -0+689
(SPAN 1x110.0M) (SHEET-04 OF 04)

SCALE : AS SHOWN PAPER SIZE : A2

DRG. NO. STR/DWG/MJB/SUPER-STRUCTURE-301			
DATE : SEPTEMBER, 2022		REV: R0	
DESIGNED GS	DRAWN SP	CHECKED AS	APPROVED SM



NATIONAL HIGHWAYS AND INFRASTRUCTURE DEVELOPMENT CORPORATION

Building, 4 Parliament Street, Sansad Marg Area, New Delhi- 110001



EPC Contractor

Beigh Construction Company Pvt. Ltd.
Plot No : 4, Wave Mall, Bypass Road,
Narwal Jammu

Construction of Bi Directional Tunnel at " KM 83" of length 495m including Approach Roads of 664 m and a Major Bridge of 110m from Km 82 + 675 to Km 82+ 925 on NH-244 in Jammu & Kashmir on EPC Mode

PAVEMENT DESIGN

21-12-2020



Central, oficinas y laboratorio: Pasaje Bisbal,
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☎(+91)194-2310344, ✉space@seclindia.com

**CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT "KM 83" OF LENGTH 495m APPROACHES ROAD OF 664M AND A MAJOR BRIDGE OF 110M
FROM KM 82+675 TO KM 82+925 ON NH-244 IN Jammu& Kashmir ON EPC MODE**

IRC:58-2015 Guidelines of		
Plain jointed Rigid Pavements for Highways		
Design of Slab thickness for pavement		
Criteria : Concrete pavement bounded with Dry lean concrete		
Specification: Two Lane Two Way(2L2W)-Lane width : 3.5 m : Spacing of Contraction Joint : 4.5m		
Design Parameters	Value adopted with units	Reference
A. Modulus subgrade Reaction		
CBR (Compacted Subgrade)	15%	Site Data
Modulus of subgrade-(strength of subgrade)	62MPa/m	Table-2(IRC:58-2015)
Thicknness of granular base (I&II)	200mm	Contract Document
DLC sub base thickness (adopted) (M-6.5 to M7.5) min strength 7N/mm after 7 days	150mm	Contract Document
Effective Modulus of subgrade (subgrade+GSB+DLC) -Interploation	293MPa/m	Table-4(IRC:58-2015)
B. Selection of Flexural Strength		
28 days compressive strength of Cement concrete for PQC	> 40 Mpa (Target strength after 90days 48MPa) $F_t = F_{ck} + 1.65 \cdot 5$	IS:10262 (2009) & IS :456200
28 days flexural strength of Cement concrete for PQC	4.4 to 4.5 Mpa (4.84 Mpa after 90days)	IS:10262 (2009) & IS :456200
C. Design Traffic criteria:		
Design Period	30 years	IRC:58-2015/ Contract document
Design Traffic	20msa	Contract Document
Hence, design number of axle load repetitions for bottom-up cracking analysis = 20000000		
D. Cumulative Fatigue Damage (CFD) analysis for Bottom-Up Cracking (BUC)and Top-Down Cracking (TDC) and Selection of Slab Thickness		
Effective modulus of subgrade reaction of foundation, k	62MPa/m	
Elastic Modulus of concrete, E	28000 Mpa	
Poisson's ratio of concrete, μ	0.15	
Unit weight of concrete, γ	25 kN/m3	
Design flexural strength of concrete	4.8 Mpa	
Max. day-time Temperature Differential in slab (for bottom-up cracking)	15.8 Degree C	Table-1(IRC:58-2015) for thickness range of 300mm-400mm (location : J&K)
Night-time Temperature Differential in slab (for top-down cracking) =day-time diff/	12.9 Degree C	
Lateral placement factor (0.25 for 2 lane 2 way)	0.25	
Factor for selction of traffic for BUC analysis (for six hours period during day)	0.25	
Other Details		
Shoulders : Tied concrete shoulders	Yes (paved shoulders)	

Flextural strength of Concrete, Mpa				4.4145	Spacing of Contraction Joints, m		4.0	
Effective modulus of subgrade reaction of DLC subbase, MN/m3				293	Width of slab, m		3.5	
Elastic Modulus of Concrete, MPa				2.943E+04	No of lanes in both directions		2	
Poisson's ratio				0.15	Type of carriageway		Divided	
Coefficient of thermal expansion of concrete, / 0C				1.00E-05	Trial Thickness of PQC, mm		290	
Tyre pressure, kPa				800	Radius of relative stiffness, m		0.676	
Growth rate of traffic				7.5%	Growth Factor		103.40	
Design Life, Years				30	Lane Distribution Factor		0.75	
Proportion of trucks plying along edge				0.34	Directional Distribution Factor		0.54	
Axle Load, t		LF*Axe Load, t	No. of Axles	stress, kg/cm2	Expected No.	Stress Ratio	Allowable No.	Damage Ratio
Single Axle With Single Wheel								
16	19	0	16.7	0	0.37	Unlimited	0.0	
14	17	8	15.0	42891	0.33	Unlimited	0.0	
12	14	21	13.4	111162	0.30	Unlimited	0.0	
10	12	71	11.6	370668	0.26	Unlimited	0.0	
8	10	501	9.8	2604818	0.22	Unlimited	0.0	
6	7	1536	7.8	7982281	0.17	Unlimited	0.0	
4	5	2093	5.7	10879141	0.13	Unlimited	0.0	
2	2	274	3.3	1426296	0.07	Unlimited	0.0	
Single Axle with Dual Wheels								
24	28.8	2	24.73	12690	0.55	126167	0.101	
22	26.4	2	23.26	12690	0.52	367648	0.035	
20	24	158	21.72	819029	0.48	2004954	0.409	
18	21.6	43	20.17	221403	0.45	90733912	0.002	
16	19.2	184	18.45	956728	0.41	Unlimited	0	
14	16.8	552		2869137	0.00	Unlimited	0	
12	14.4	890		4627569	0.00	Unlimited	0	
10	12	437		2273248	0.00	Unlimited	0	
Tandem Axle								
40	48	8	16.76	42891	0.37	Unlimited	0	
36	43.2	86	15.53	446756	0.35	Unlimited	0	
32	38.4	88	14.23	458984	0.32	Unlimited	0	
28	33.6	237		1231501	0.00	Unlimited	0	
24	28.8	638		3317578	0.00	Unlimited	0	
20	24	745		3870601	0.00	Unlimited	0	
16	19.2	291		1514834	0.00	Unlimited	0	
Tridem Axle								
48	57.6	15	21.54	78871	0.48	2646545	0.030	
44	52.8	207	20.06	1073331	0.45	151766641	0.007	
40	48	45	18.55	235089	0.41	Unlimited	0.000	
							0.58	

**CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT "KM 83" OF LENGTH 495m APPROACHES ROAD OF 664M AND A MAJOR BRIDGE OF 110M
FROM KM 82+675 TO KM 82+925 ON NH-244 IN Jammu & Kashmir ON EPC MODE**

Since summation CFD is less than 1, i.e 0.58, Hence safe n ok

I. Thickness Design:

Considering pavement option as Concrete bound pavement bounded to Dry lean concrete layer (DLC), 7days 9-10 MPa strength, DLC is imperative for the said case of Rigid pavement. Cement content in Dry lean concrete must not be less than 170 kg/cum. Separation membrane is recommended to be placed between the base and the concrete slab (PQC) to make the interface condition smooth. A de-bonding interlayer of polythene sheet white or transparent or any other colour having a minimum thickness of 125 micron is recommended as per the current practice in India. Granular sub base of 200mm thickness above sub grade is recommended.

Assuming that doweled transverse joints and tied concrete shoulders will be provided, the thickness of slab required for the given traffic conditions and other design data = 0.30m

$E(\text{Concrete}) = 30000 \text{ MPa}$ $E(\text{DLC}) = 13600 \text{ MPa}$; Poisson's ratio (con) = 0.15; Poisson's Ratio (sub) = 0.20

Provide DLC thickness of 0.15m (DLC of 9-10 MPa strength at 7days is considered)

Depth of neutral axis (computed using equation 11 of IRC:58-2015) $d = 0.19\text{m}$

$$d = \frac{0.5(h_1^2) + \left(\frac{E_2}{E_1}\right)h_2(h_1 + 0.5h_2)}{h_1 + \left(\frac{E_2}{E_1}\right)h_2} \quad \dots 11$$

Flexural Stiffness of slab proposed to be placed over DLC (Equation 12 of IRC:58-2015) = 83.78 MN.m

$$\text{Flexural stiffness (1, PQC)} = \frac{E_1 \left(\left(\frac{h_1^3}{12} \right) + h_1(d - 0.5h_1)^2 \right)}{1 - \mu_1^2} \quad \dots 12$$

Flexural Stiffness of DLC (Equation 13 of IRC:58-2015) = 34.77 MN.m

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$$\text{Flexural Modulus (2, DLC)} = \frac{E_2 \left[\frac{\left(\frac{E_2}{E_1} \right) h_2^3}{12} + \left(\frac{E_2}{E_1} \right) h_2 \left(h_1 + \frac{h_2}{2} - d \right)^2 \right]}{(1 - \mu_2^2)} \quad \dots (13)$$

Combined stiffness of Slab and DLC = 118.55 MN.m

Stiffness of design slab of 0.3m thickness (Equation 10 of IRC:58-2015) = 69.05 MN.m

$$\frac{EI}{1 - \mu^2} = \frac{Eh^3}{12(1 - \mu^2)} \quad \dots (10)$$

Combined stiffness is more than Design Stiffness requirement. Hence Ok :

Conclusion: Adopt 300mm thickness of PQC with 150 mm DLC and 200mm GSB

II. Design of Dowel bars:

Slab thickness : 300mm

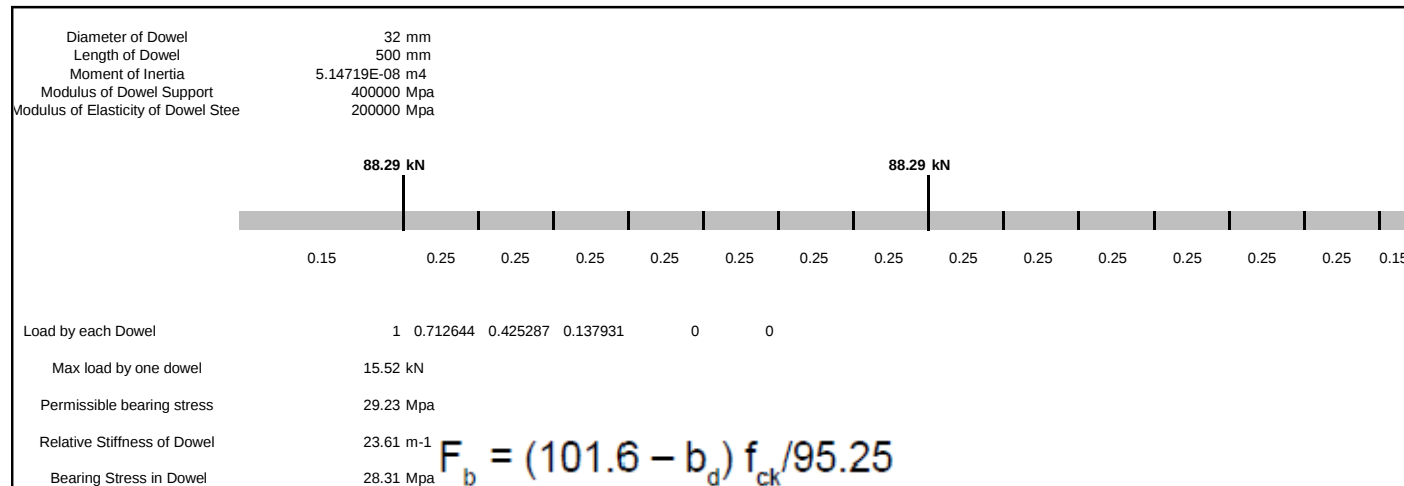
Joint width = 20mm

Radius of Relative stiffness = 1035.3 mm

E for dowel bar = 2×10^5 MPa

Modulus of dowel support, $k_{\text{mds}} = 415000$ MPa/m

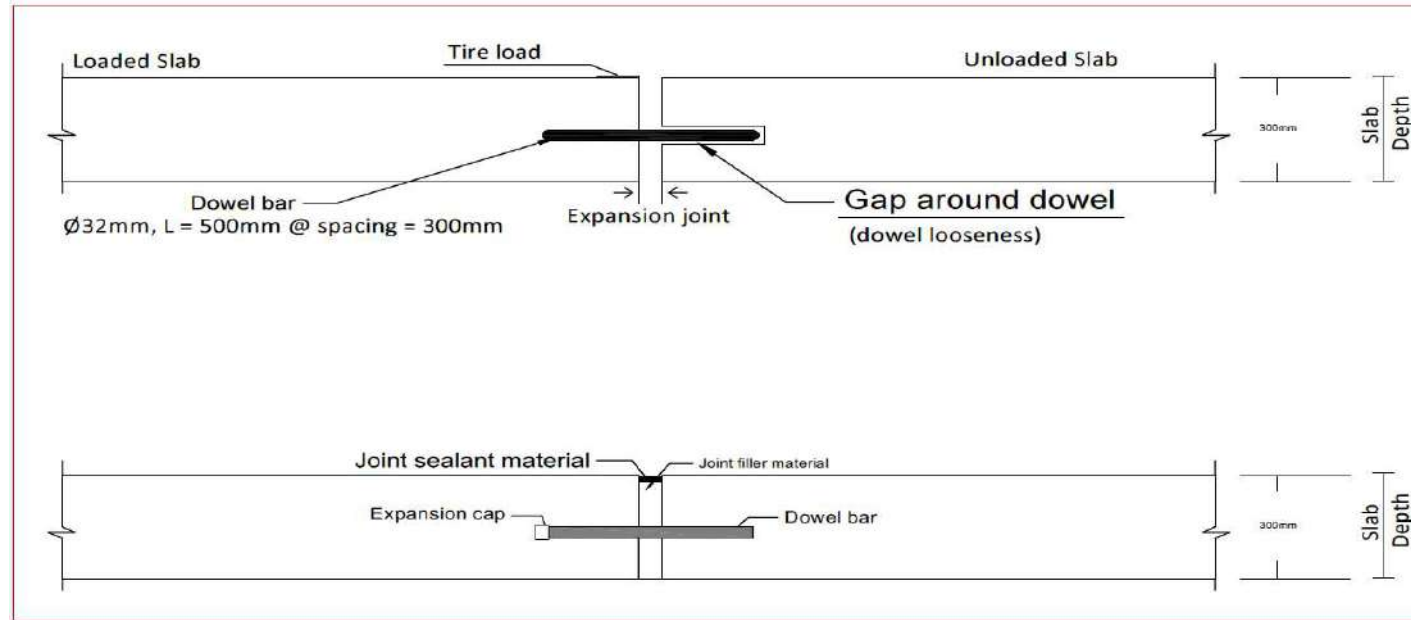
**CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT “KM 83” OF LENGTH 495m APPROACHES ROAD OF 664M AND A MAJOR BRIDGE OF 110M
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Since Bearing stress in dowel bar is less than permissible bearing stress in concrete computed using e.15 IRC:58-2015.

Dowel bars of dia 32 mm, with length 500 mm and spacing 300mm is recommended for the current rigid pavement .

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III. Check for Corner stresses:

Corner stresses have been computed. The maximum stress (bending tension) σ_c and deflection δ_c due to the load at corner of a rigid slab is given by Westergaard as:

$$\sigma_c = \frac{3P}{h^2} \left[1 - \left(\frac{a\sqrt{2}}{\ell} \right)^{0.6} \right]$$

$$\delta_c = \frac{P}{k\ell^2} \left[1.1 - 0.88 \left(\frac{a\sqrt{2}}{\ell} \right) \right]$$

Where:
 k = modulus of subgrade reaction
 ℓ = radius of relative stiffness
 a = load contact radius
 P = load

The above equation for σ_c as modified by Kelly is being used by IRC for computing the wheel load stress at corner

**CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT "KM 83" OF LENGTH 495m APPROACHES ROAD OF 664M AND A MAJOR BRIDGE OF 110M
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$$\sigma_c = \frac{3P}{h^2} \left[1 - \left(\frac{a\sqrt{2}}{l} \right)^{1.2} \right]$$

And is worked out as : Computed using Excel program

:

Check for Corner Stress	
Axle Load, t	24
Wheel Load, t	12
Radius of contact, cm	30.59
Corner Stress, kg/cm ²	17.30

Since , Corner stress is less than Flexural strength of concrete and the pavement thickness of 300mm assumed is **Safe**

IV. Design of Tie Bars

Area of steel required per metre length of joint has been computed using the following equation:

$$A_s = \frac{b f w}{S_{st}} \quad \dots (16)$$

In which,

A_s = area of steel in mm², required per m length of joint

b = lane width in metres

f = coefficient of friction between pavement and the subbase/base (usually taken as 1.5)

W = weight of slab in kN/m² and

S_{st} = allowable working stress of steel in MPa

As per the provisions of IRC:58-2015, The length of any tie bar should be at least twice that required to develop a bond strength equal to the working stress of the steel. The formula for estimating the length of tiebar is given as equation 17.

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$$L = \frac{2 S_{st} A_{cs}}{B^* P_{ptb}} \quad \dots (17)$$

In which:

- L = length of tie bar (mm)
- S_{st} = allowable working stress in steel (MPa)
- A_{cs} = cross-sectional area of one tie bar (mm²)
- P_{ptb} = perimeter of tie bar (mm), and
- B^* = permissible bond stress of concrete (i) for deformed tie bars = 2.46 MPa,
(ii) for plain tie bars – 1.75 MPa.

- (i). Slab thickness = 0.30 m
- (ii). Lane width (b) = 3.5 m
- (iii). Coefficient of Friction (f) = 1.5
- (iv). Density of concrete (KN/m³) = 24
- (v). Allowable Tensile stress in deformed bar, S_{st} (MPa) = 200 (IRC:15-2011)
- (vi). Allowable bond stress in deformed bar (B^*) = 2.46
- (vii). Dia of tie bar = 12 mm
- (viii). Area of Tie bar = 113.1 mm²

Spacing and Length of Deformed bars: Using Excel developed programme using above equations and parameters

Tie bar design		Deformed bars
Area of tie bar steel, cm ²	3.024	1.89
Area of 12 mm tie bar	1.131	1.131
Perimeter of tie bar	3.770	3.770
Spacing, cm	37.400	59.840
Length of tie bar, cm	57.857	63.780

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Design Conclusion:

- I. Provide pavement thickness of 300mm Pavement Quality Concrete (PQC) having 28-days Flexural and Compressive Strengths 4.5 MPa and 40 MPa respectively over 150 mm Dry Lean Concrete Subbase with a minimum 7-Day Compressive Strength of 7 MPa with 32 mm diameter Dowel bars of length 500 mm at a spacing of 300mm and deformed Tiebars of 12 mm diameter, 640 mm length at a spacing of 600 mm (c/c).
- II. Provide a drainage layer (GSB) of 200 mm thickness with a drainage coefficient of min. 350 m per day.

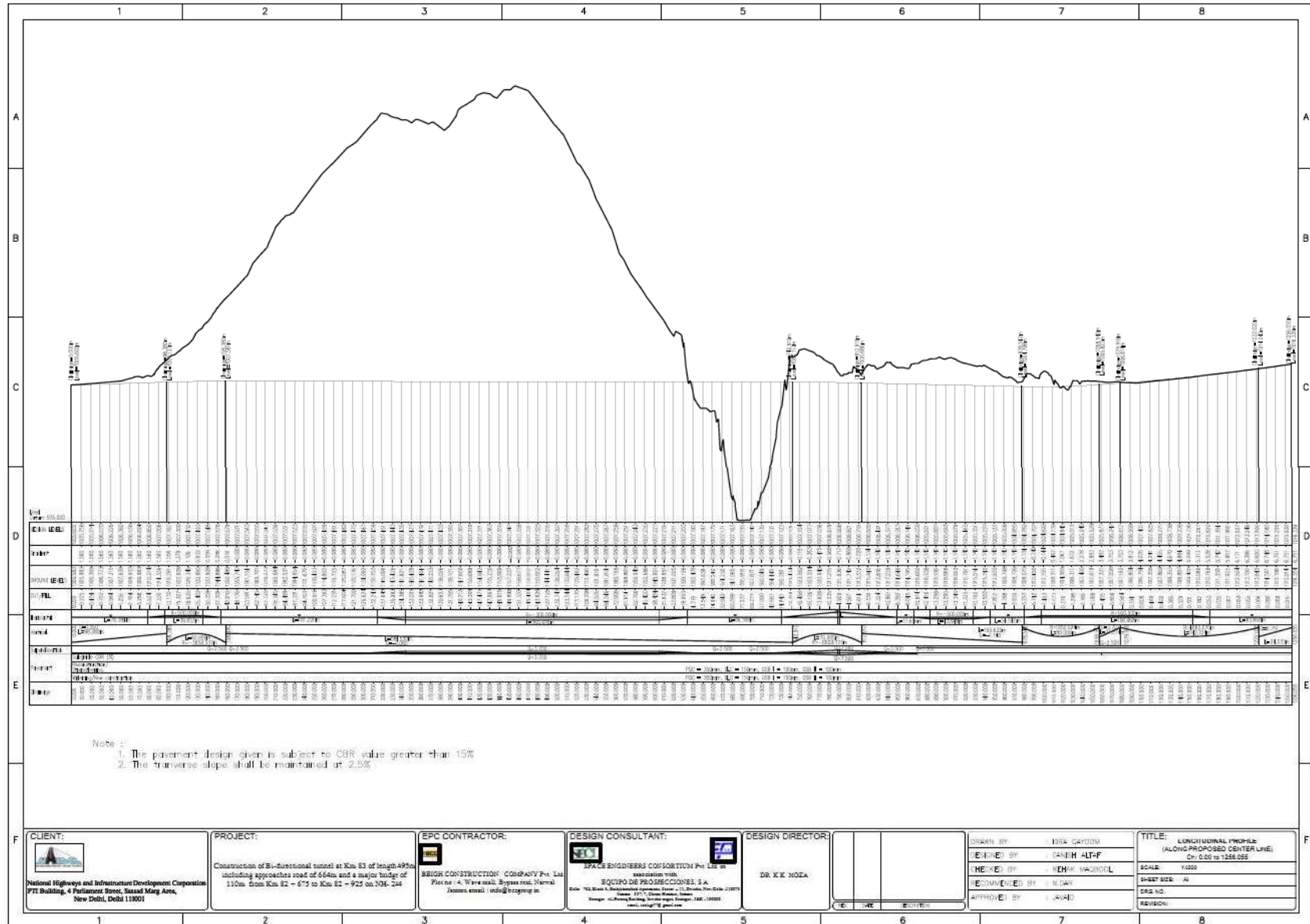
CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT “KM 83” OF LENGTH 495m APPROACHES ROAD OF 664M AND A MAJOR BRIDGE OF 110M FROM KM 82+675 TO KM 82+925 ON NH-244 IN Jammu& Kashmir ON EPC MODE

Roadside Drainage

S. No	DESIGN CHAINAGE		LENGTH	DRAIN	TCS TYPE
	From	To			
01	0+000	0+065	65	Both Sides	TCS-1
02	0+065	0+128	63	Both Sides	TCS-1
03	0+733	0+745	12	One Side	TCS-3
04	0+745	0+780	35	Both Sides	TCS-1
05	0+780	0+845	65	One Side	TCS-2
06	0+845	0+880	35	One Side	TCS-3
07	0+880	0+935	55	One Side	TCS-3
08	0+935	0+975	40	One Side	TCS-3
09	0+975	1+015	40	One Side	TCS-2
10	1+015	1+080	65	One Side	TCS-3
11	1+080	1+155	75	One Side	TCS-2
12	1+155	1+215	60	One Side	TCS-1
13	1+215	1+240	25	Both Sides	TCS-2
14	1+240	1+269	29	One Side	TCS-3

CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT "KM 83" OF LENGTH 495m APPROACHES ROAD OF 664M AND A MAJOR BRIDGE OF 110M FROM KM 82+675 TO KM 82+925 ON NH-244 IN Jammu& Kashmir ON EPC MODE

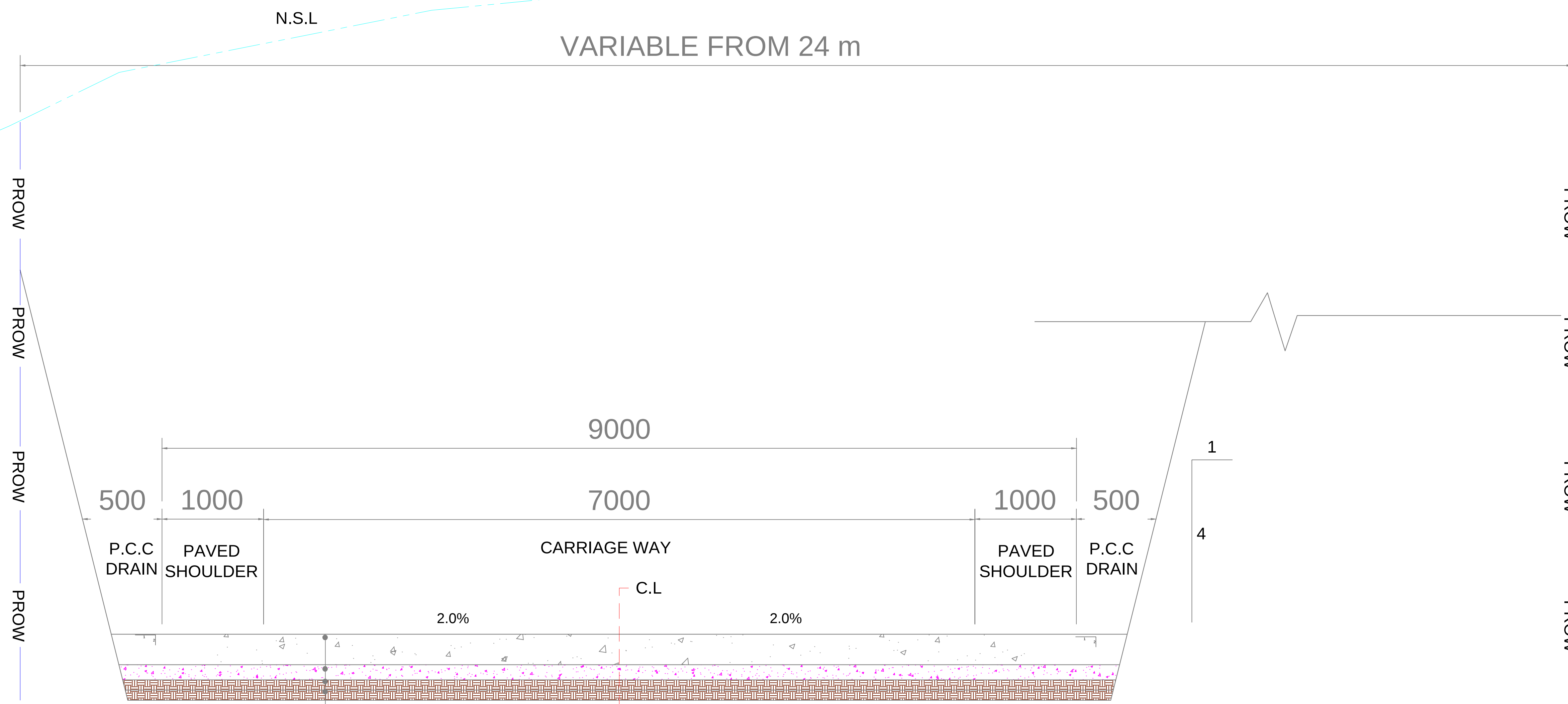
III.



**CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT “KM 83” OF LENGTH 495m APPROACHES ROAD OF 664M AND A MAJOR BRIDGE OF 110M
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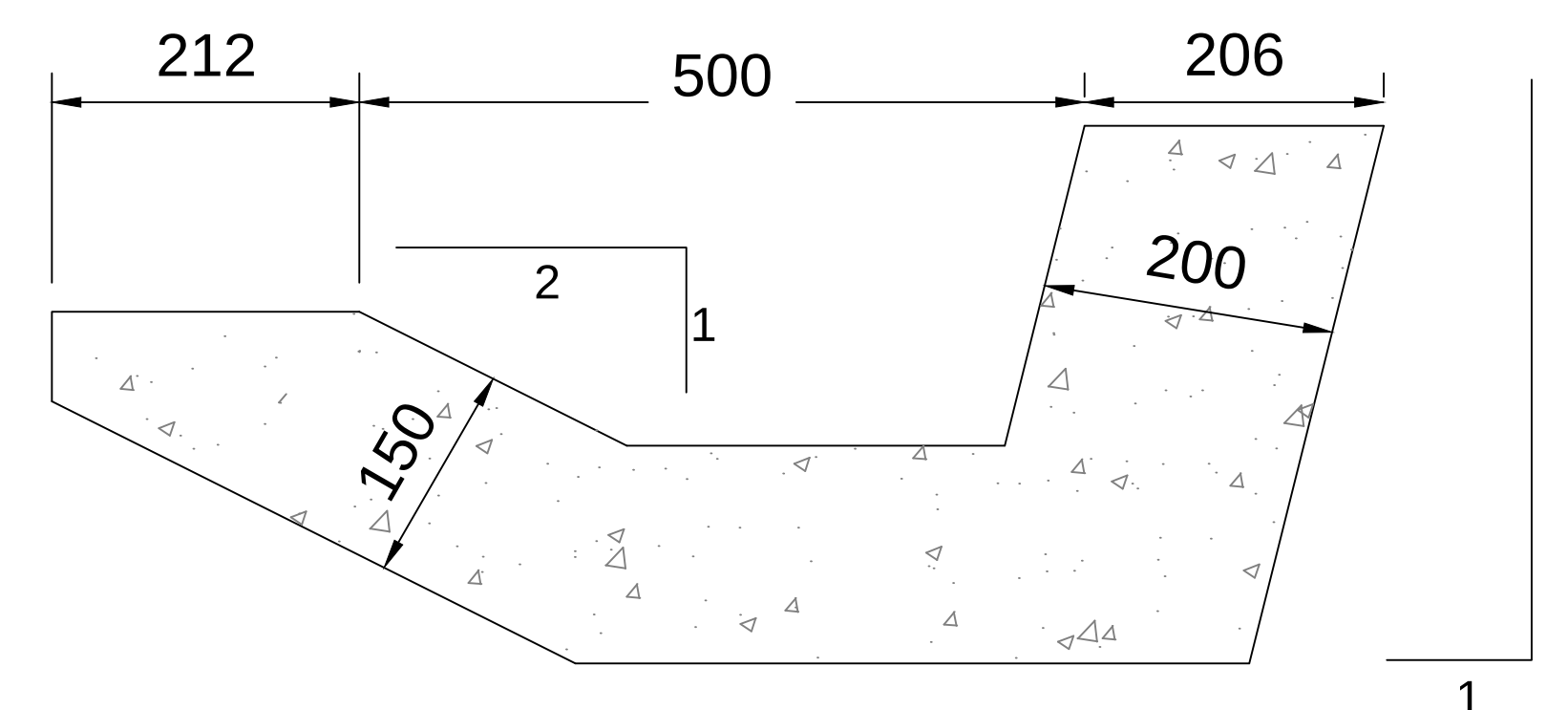
Pavement Quality Concrete (PQC)300 mm
Dry lean concrete (DLC)150mm
GSB -I - 100 mm
GSB -II - 100 mm
Sub grade

CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT “KM 83” OF LENGTH 495m APPROACHES ROAD OF 664M AND A MAJOR BRIDGE OF 110M FROM KM 82+675 TO KM 82+925 ON NH-244 IN Jammu& Kashmir ON EPC MODE



- 300mm. PAVEMENT QUALITY CONCRETE
- 150 mm. DRY LEAN CEMENT CONCRETE BASE LAYER
- 100mm. GSB I
- 100mm. GSB II

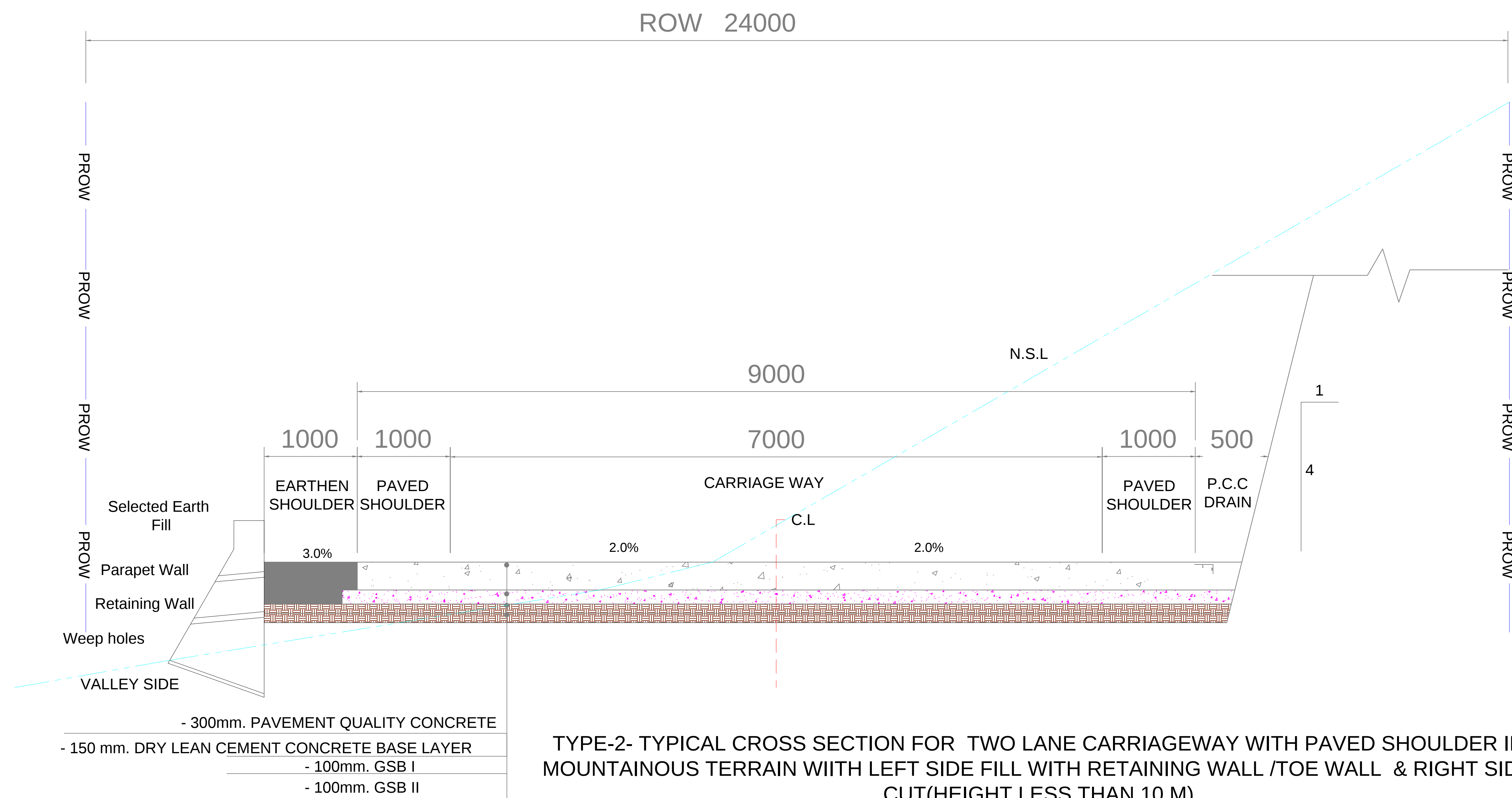
TYPE-1- TYPICAL CROSS SECTION FOR TWO LANE CARRIAGEWAY WITH PAVED SHOULDER IN MOUNTAINOUS TERRAIN WITH BOTH SIDE CUT



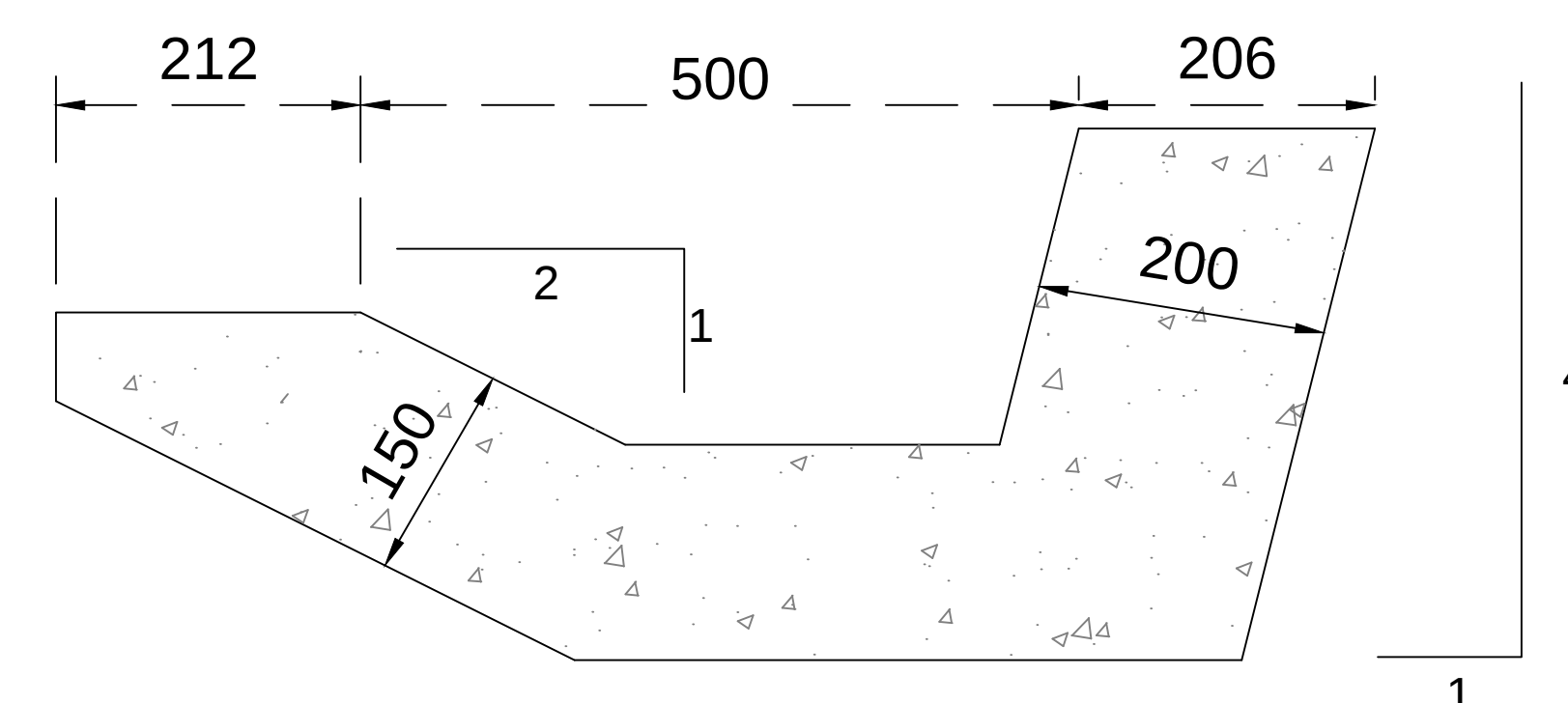
DETAILS 1

NOTE:
1.All dimensions are in mm unless otherwise mentioned.

CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT “KM 83” OF LENGTH 495m APPROACHES ROAD OF 664M AND A MAJOR BRIDGE OF 110M FROM KM 82+675 TO KM 82+925 ON NH-244 IN Jammu& Kashmir ON EPC MODE

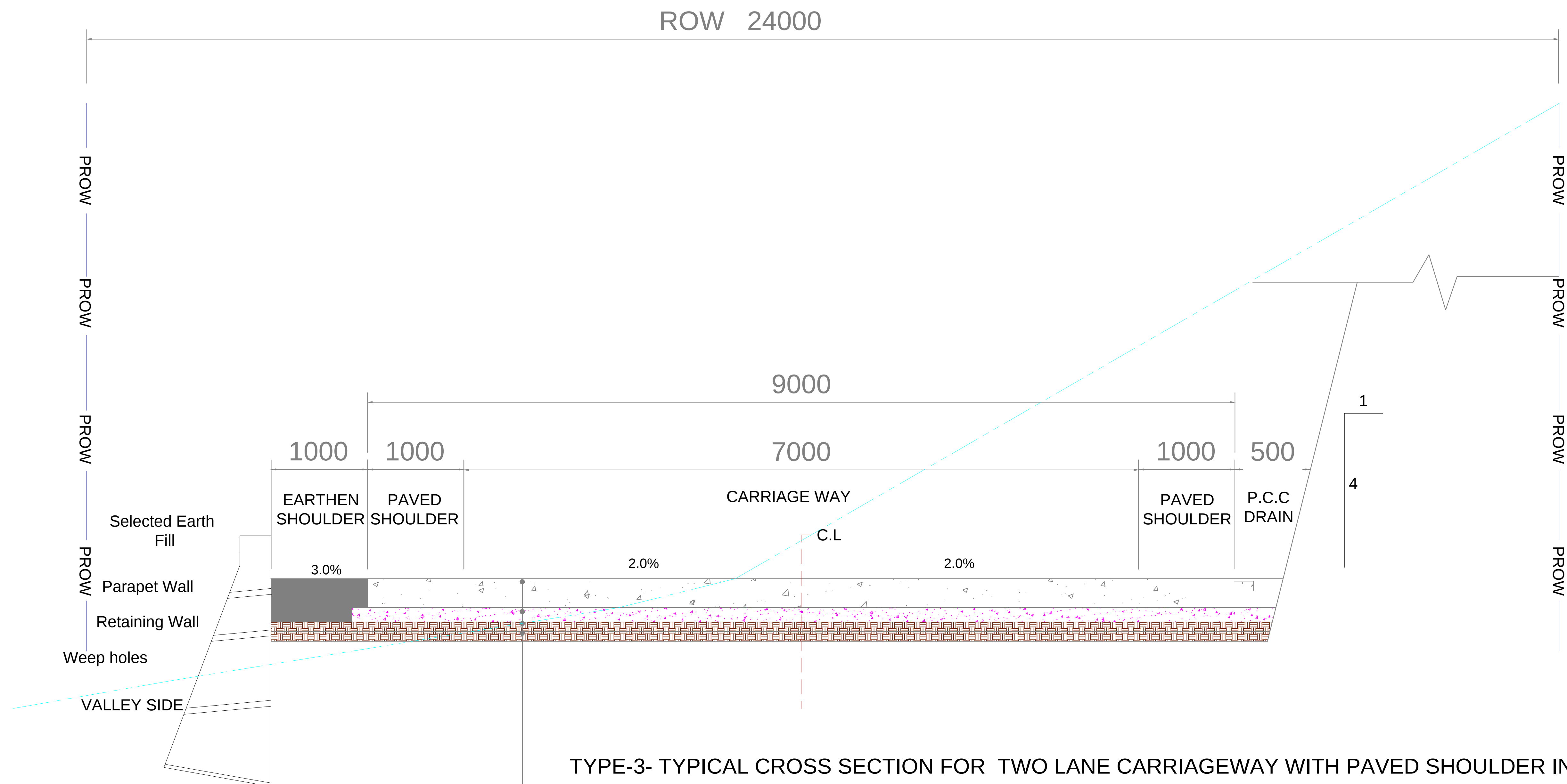


TYPE-2- TYPICAL CROSS SECTION FOR TWO LANE CARRIAGEWAY WITH PAVED SHOULDER IN MOUNTAINOUS TERRAIN WITH LEFT SIDE FILL WITH RETAINING WALL /TOE WALL & RIGHT SIDE CUT(HEIGHT LESS THAN 10 M)



NOTE:
1.All dimensions are in mm unless otherwise mentioned.

CONSTRUCTION OF BI-DIRECTIONAL TUNNEL AT “KM 83” OF LENGTH 495m APPROACHES ROAD OF 664M AND A MAJOR BRIDGE OF 110M FROM KM 82+675 TO KM 82+925 ON NH-244 IN Jammu & KASHMIR ON EPC MODE



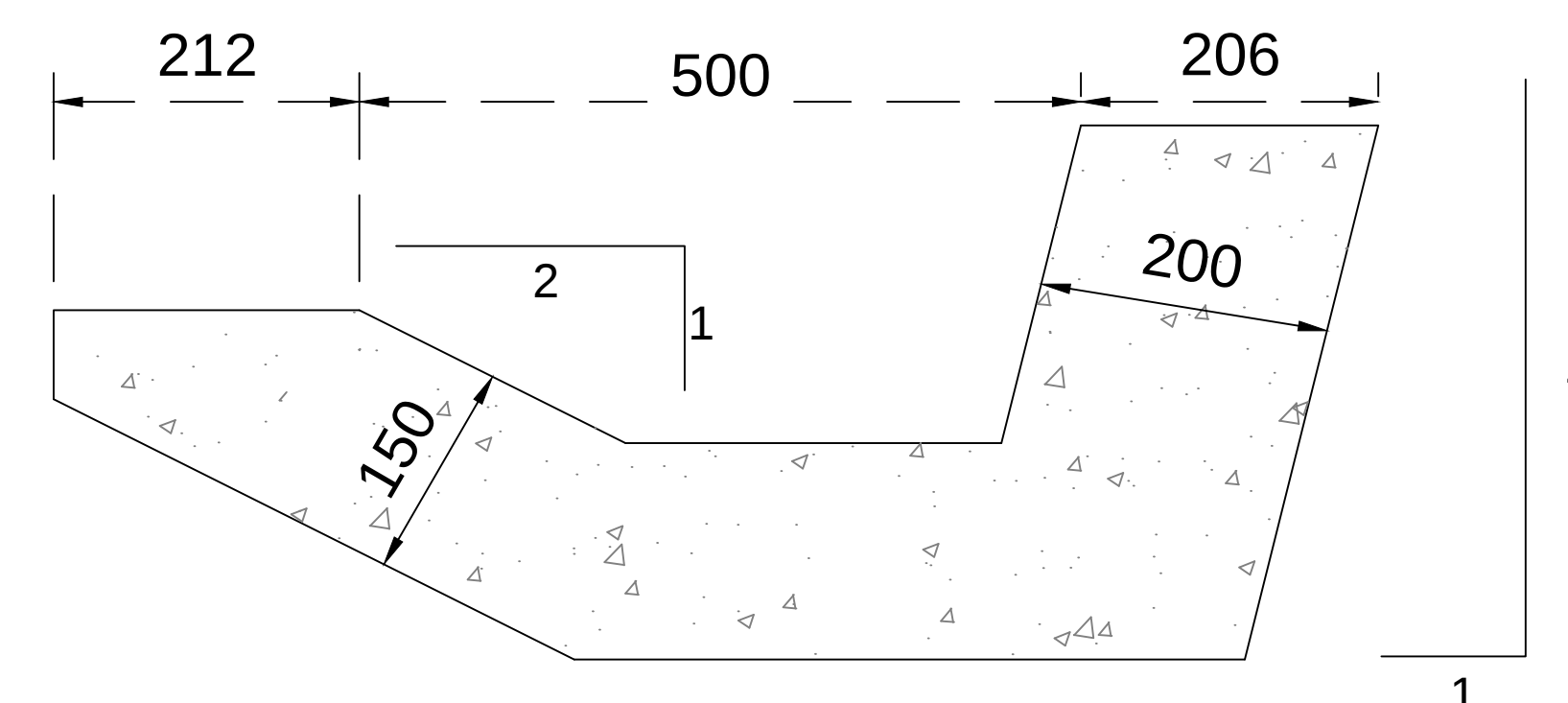
TYPE-3- TYPICAL CROSS SECTION FOR TWO LANE CARRIAGEWAY WITH PAVED SHOULDER IN MOUNTAINOUS TERRAIN WITH LEFT SIDE FILL WITH RETAINING WALL /TOE WALL & RIGHT SIDE CUT(HEIGHT MORE THAN 10 M)

- 300mm. PAVEMENT QUALITY CONCRETE

- 150 mm. DRY LEAN CEMENT CONCRETE BASE LAYER

- 100mm. GSB I

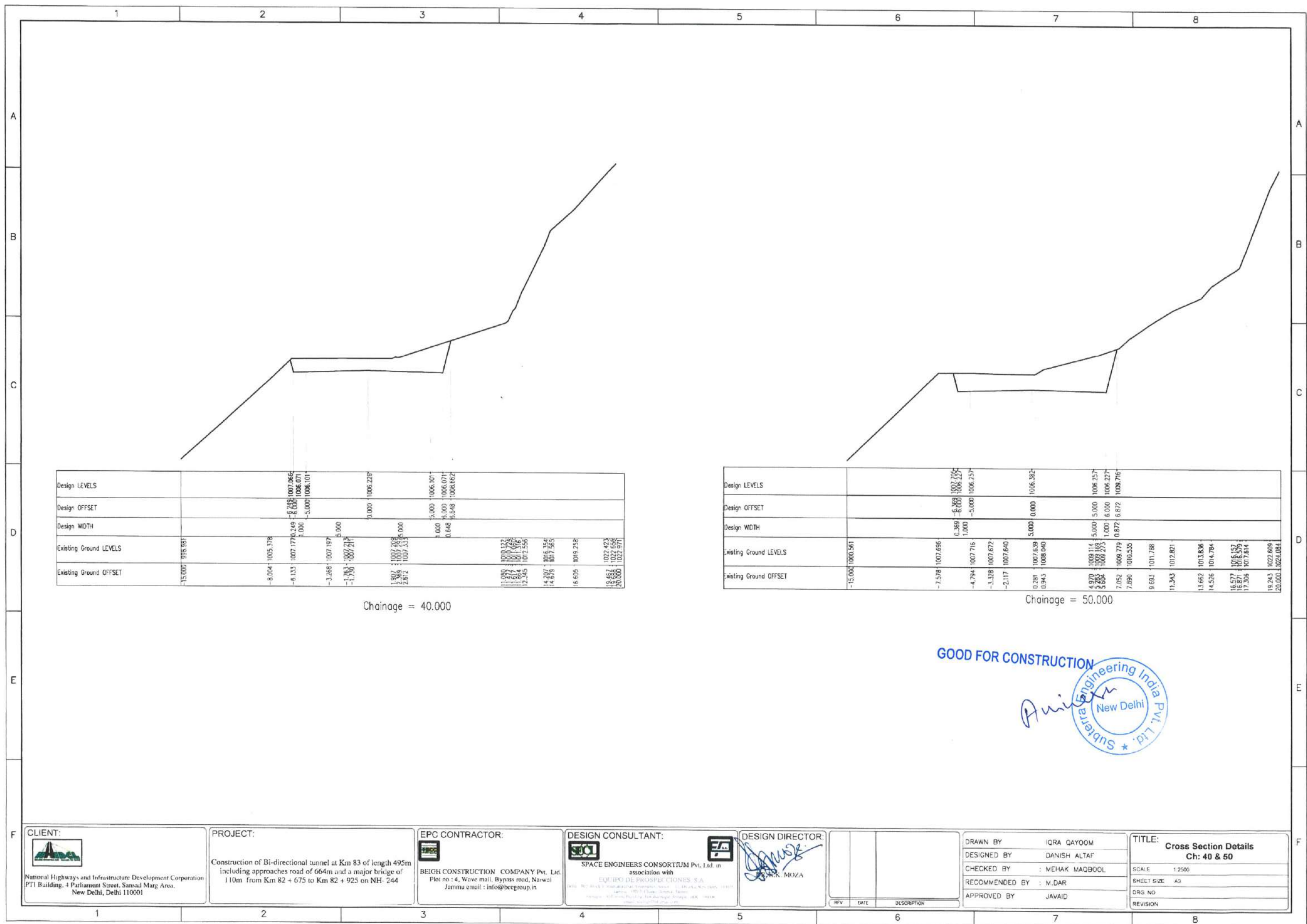
- 100mm. GSB II

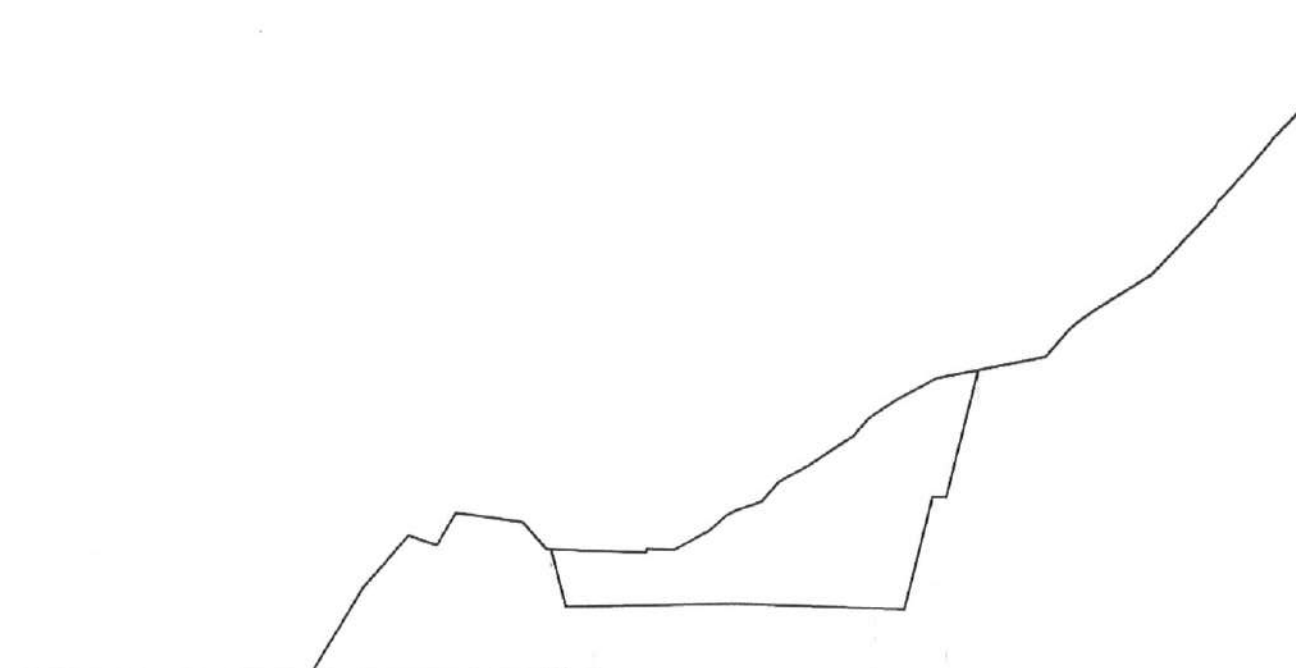


NOTE:

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DETAILS 1





Existing Ground OFFSET	Existing Ground LEVELS	Design WIDTH	Design OFFSET	Design LEVELS
-15.000	1004.163			
-13.188	1007.193			
-11.584	1009.033			
-10.622	1008.540			
-10.077	1008.698			
-9.914	1009.854			
-7.526	1009.536			
-6.569	1008.595			
-6.249	1008.372			
-3.140	1008.492			
-3.128	1008.516			
-2.149	1008.590			
-1.102	1009.164			
-0.784	1009.312			
-0.037	1010.002			
0.949	1010.319			
1.585	1011.046			
2.528	1011.584	5.000		
4.179	1012.685			
4.753	1013.316			
5.770	1013.988	1.000		
7.128	1014.727	1.000		
		0.500		
		1.123		
			8.623	1015.034
11.024	1015.526			
11.887	1015.521			
12.722	1015.730			
	1017.143			
14.777	1018.434			
17.074	1020.848			
17.156	1021.016			
17.624	1021.459			
19.131	1022.291			

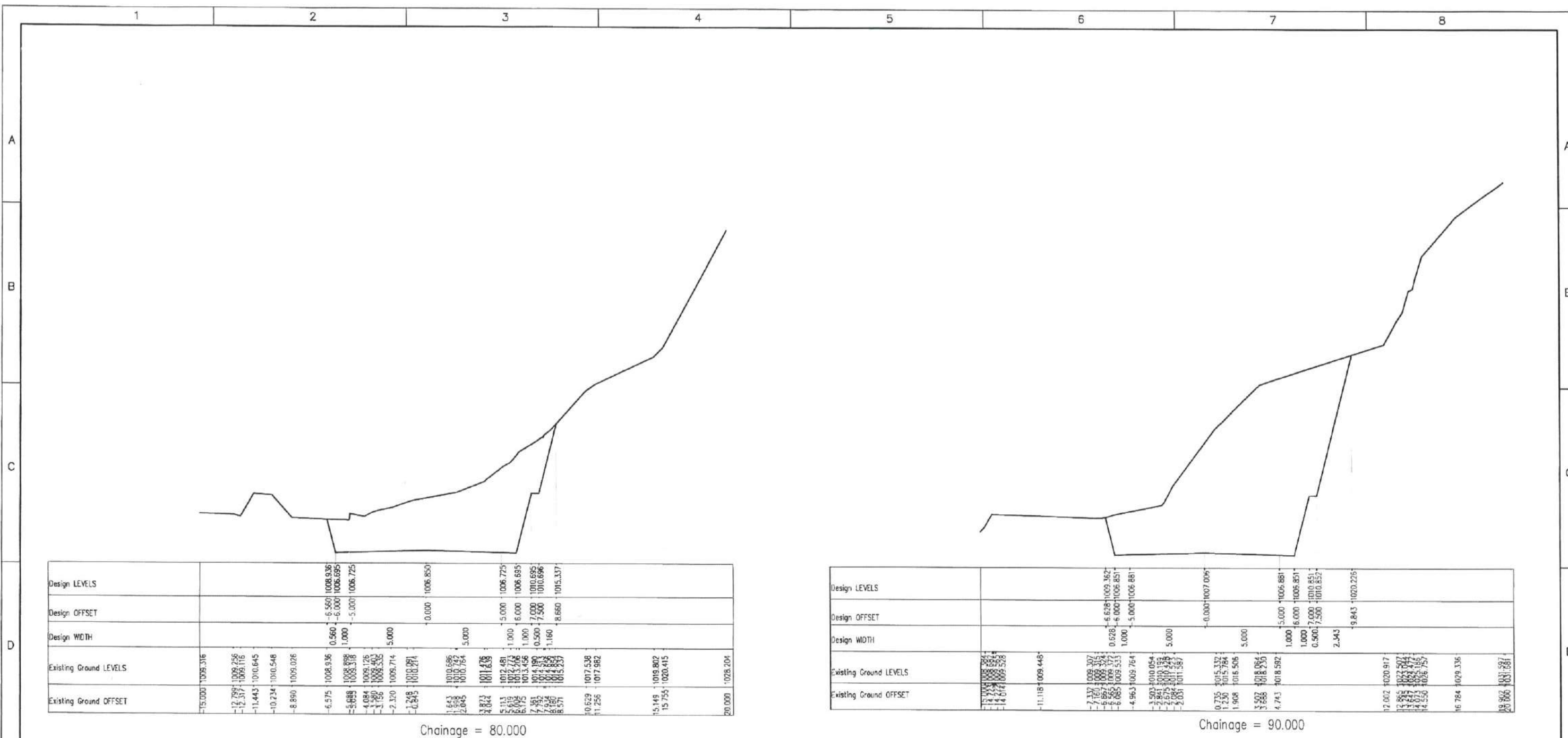
Chainage = 70.000

CONSTRUCTION

Engineering India Pvt. Ltd.
New Delhi

Subrahmanya

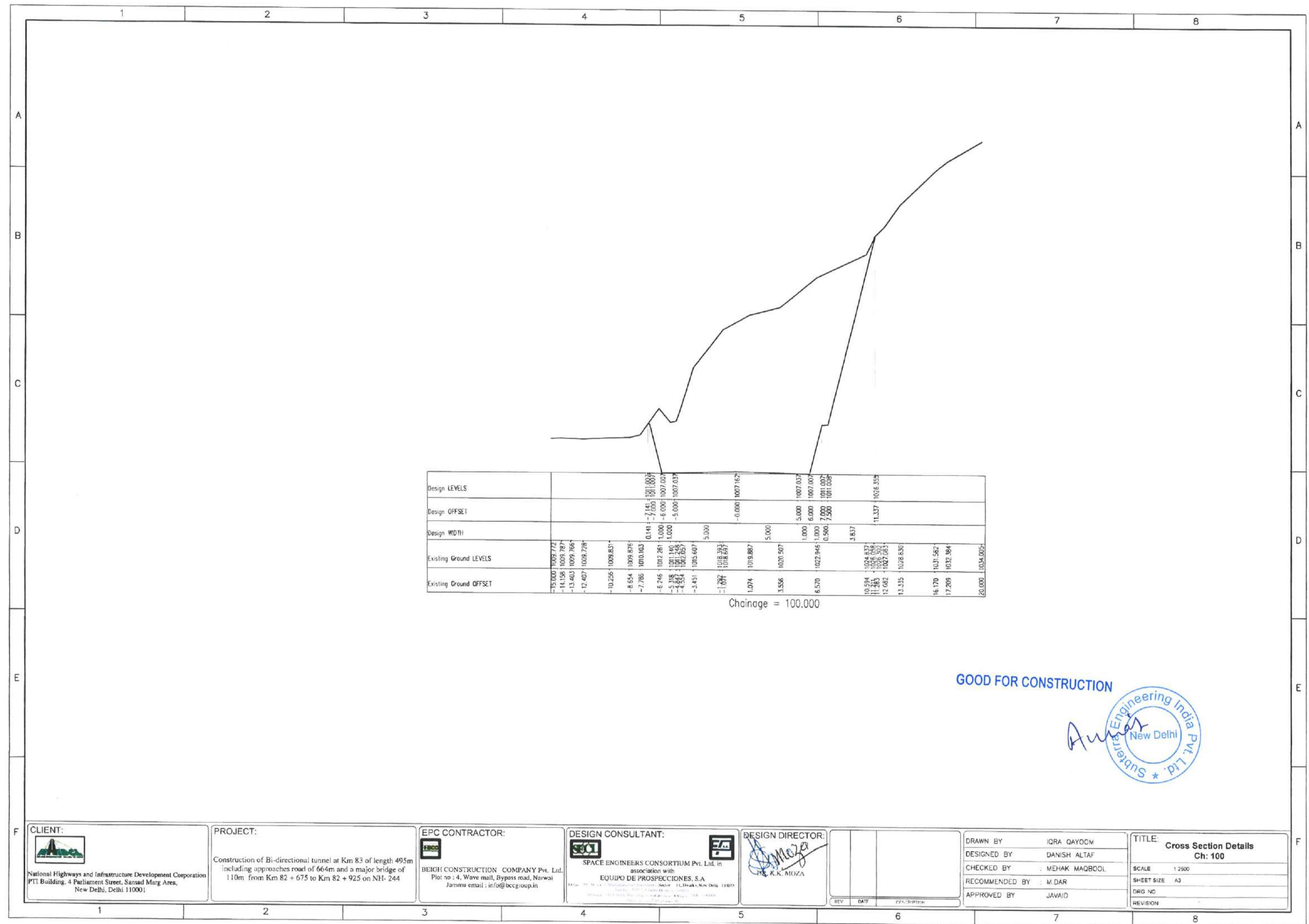
CLIENT:  National Highways and Infrastructure Development Corporation PTI Building, 4 Parliament Street, Sansad Marg Area, New Delhi, Delhi 110001	PROJECT: Construction of Bi-directional tunnel at Km 83 of length 495m including approaches road of 664m and a major bridge of 110m from Km 82 + 675 to Km 82 + 925 on NH- 244	EPC CONTRACTOR:  BEIGH CONSTRUCTION COMPANY Pvt. Ltd. Plot no : 4, Wave mall, Bypass road, Narwal Jammu email : info@bccgroup.in	DESIGN CONSULTANT:  SPACE ENGINEERS CONSORTIUM Pvt. Ltd. in association with EQUIPMENT DESIGNER CHONES, S.A. (Spain) - 902 88 11 11 / 904 04 04 04 / 904 04 04 04 - 28015 (Spain) - 91 52 11 11 / 91 52 11 11 - Madrid (France) - 01 47 88 88 88 / 01 47 88 88 88 - Paris Email : design@edc.es	DESIGN DIRECTOR:  MR. K.K. MOZA	<table border="1"> <thead> <tr> <th>REV.</th> <th>DATE</th> <th>DESCRIPTION</th> </tr> </thead> <tbody> <tr> <td> </td> <td> </td> <td> </td> </tr> </tbody> </table>	REV.	DATE	DESCRIPTION				<table border="1"> <tr> <td>DRAWN BY :</td> <td>IORA QAYOOM</td> </tr> <tr> <td>DESIGNED BY :</td> <td>DANISH ALTAUF</td> </tr> <tr> <td>CHECKED BY :</td> <td>MEHAK MAQBOOL</td> </tr> <tr> <td>RECOMMENDED BY :</td> <td>M.DAR</td> </tr> <tr> <td>APPROVED BY :</td> <td>JAVAIID</td> </tr> </table>	DRAWN BY :	IORA QAYOOM	DESIGNED BY :	DANISH ALTAUF	CHECKED BY :	MEHAK MAQBOOL	RECOMMENDED BY :	M.DAR	APPROVED BY :	JAVAIID	TITLE: Cross Section Details Ch: 60 & 70 SCALE 1:2500 SHEET SIZE A3 DRG NO REVISION
REV.	DATE	DESCRIPTION																					
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APPROVED BY :	JAVAIID																						

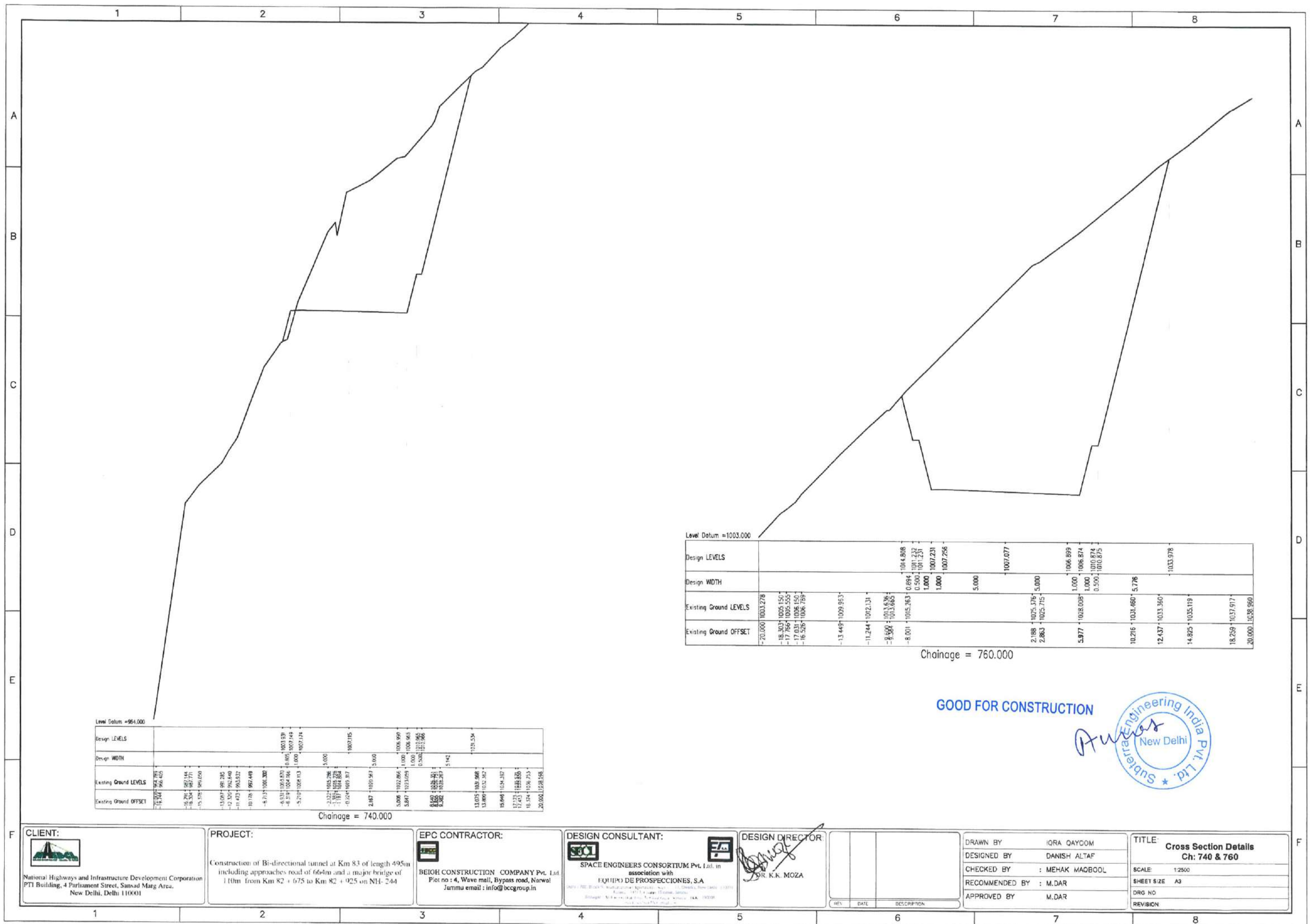


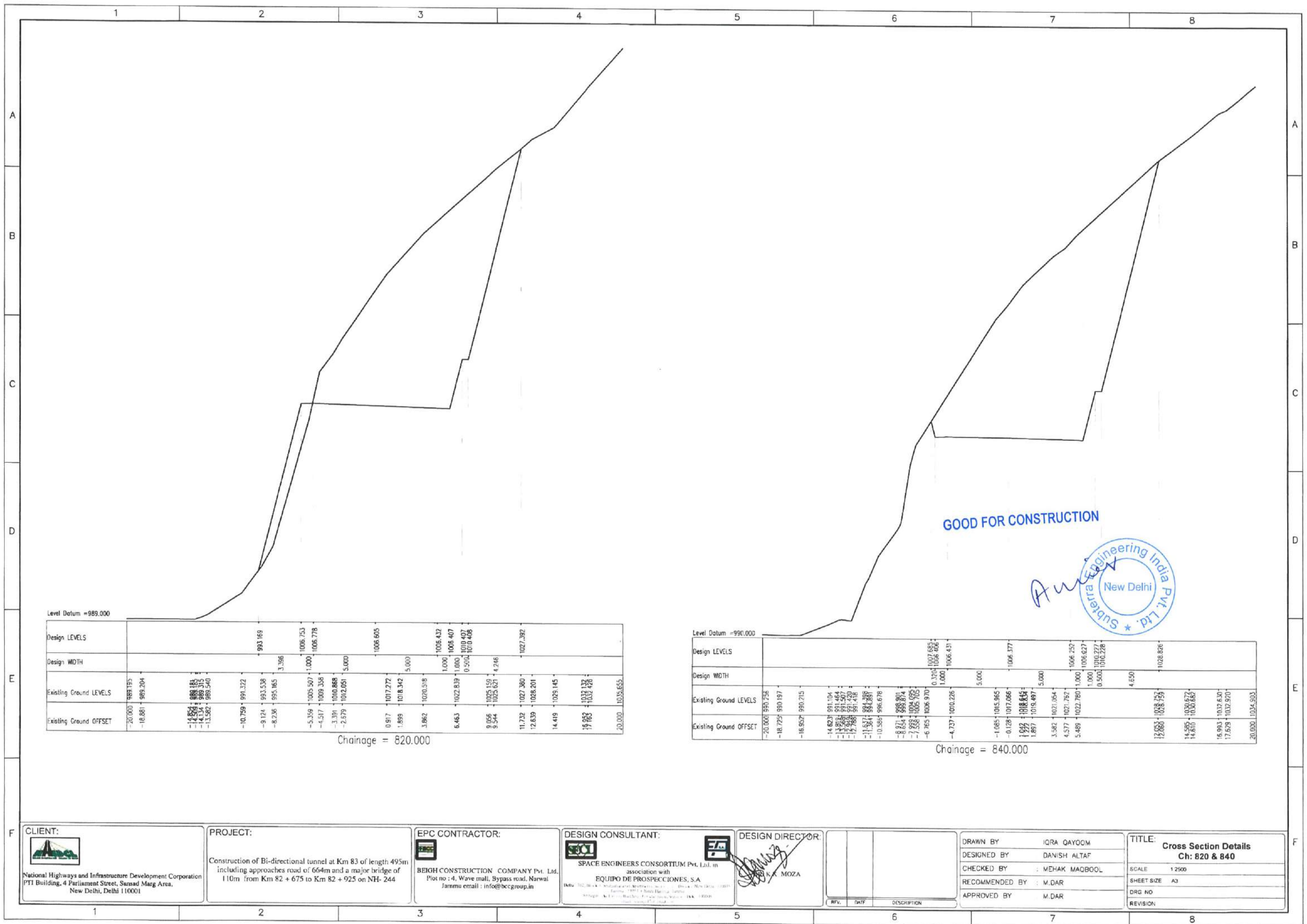
GOOD FOR CONSTRUCTION



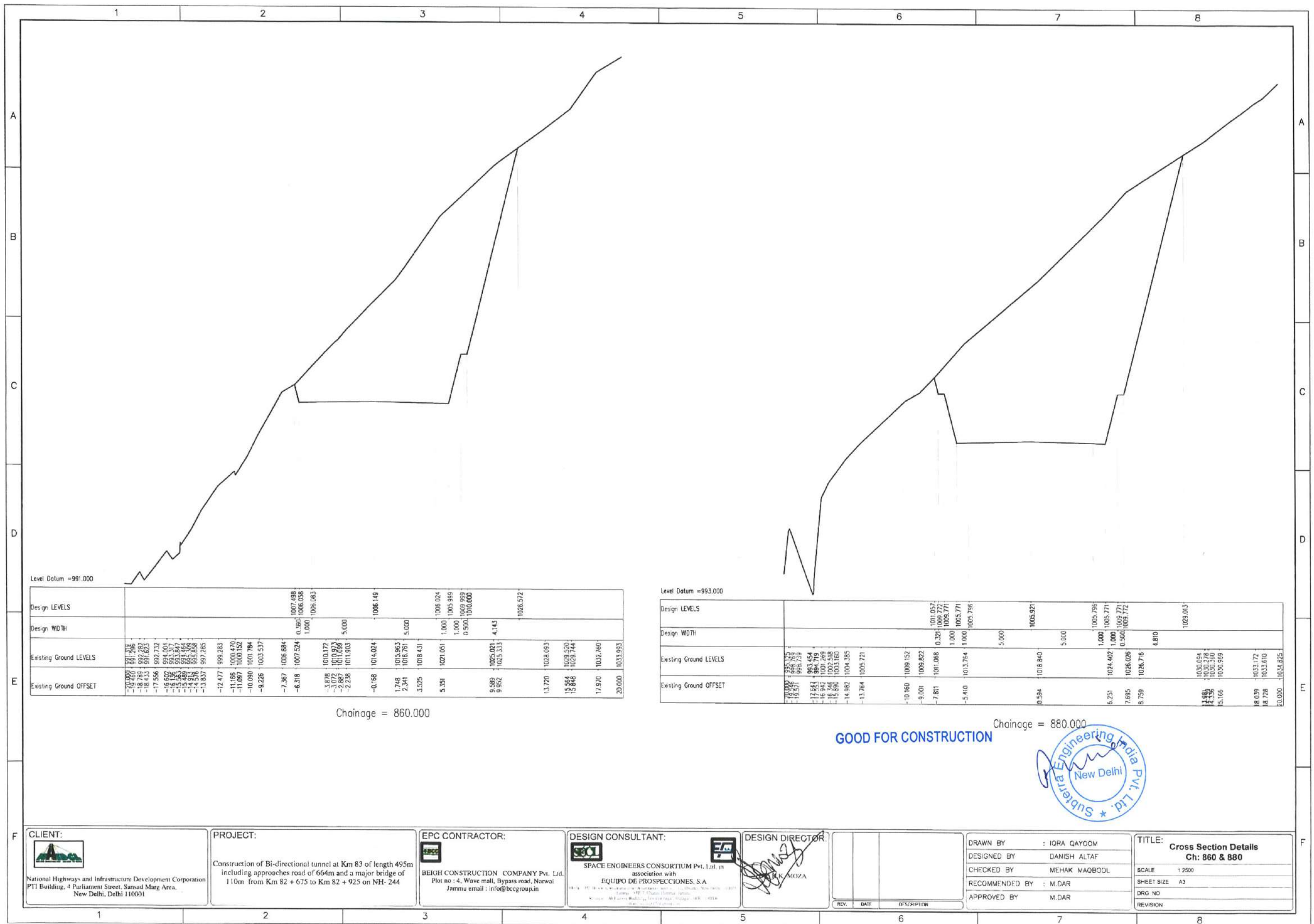
CLIENT: National Highways and Infrastructure Development Corporation PTI Building, 4 Parliament Street, Sansad Marg Area, New Delhi, Delhi 110001	PROJECT: Construction of Bi-directional tunnel at Km 83 of length 495m including approaches road of 664m and a major bridge of 110m from Km 82 + 675 to Km 82 + 925 on NH- 244	EPC CONTRACTOR: BEIGH CONSTRUCTION COMPANY Pvt. Ltd. Plot no : 4, Wave mail, Bypass road, Narwal Jammu email : info@bcgroup.in	DESIGN CONSULTANT: SPACE ENGINEERS CONSORTIUM Pvt. Ltd. in association with EQUIPO DE PROSPECCIONES, S.A. Office: 202, Block 9, Shalimar Garden Apartments, Sector - 11, Okhla, New Delhi - 110075 Jammu: 15173, 1st floor, Hemraj, Jammu Shimoga: A1-1st floor Building, 3rd stage, Shimoga, K.R. 190006 email: secon@rediffmail.com	DESIGN DIRECTOR: DR. K.K. MOZA	<table border="1"><thead><tr><th>REV</th><th>DATE</th><th>DESCRIPTION</th></tr></thead><tbody><tr><td> </td><td> </td><td> </td></tr><tr><td> </td><td> </td><td> </td></tr><tr><td> </td><td> </td><td> </td></tr></tbody></table>	REV	DATE	DESCRIPTION										<table border="1"><tr><td>DRAWN BY</td><td>IQRA QAYOUM</td></tr><tr><td>DESIGNED BY</td><td>DANISH ALTAH</td></tr><tr><td>CHECKED BY</td><td>MEHAK MAQBOL</td></tr><tr><td>RECOMMENDED BY</td><td>M.DAR</td></tr><tr><td>APPROVED BY</td><td>JAVAD</td></tr></table>	DRAWN BY	IQRA QAYOUM	DESIGNED BY	DANISH ALTAH	CHECKED BY	MEHAK MAQBOL	RECOMMENDED BY	M.DAR	APPROVED BY	JAVAD	TITLE: Cross Section Details Ch: 80 & 90 <table border="1"><tr><td>SCALE</td><td>1:2500</td></tr><tr><td>SHEET SIZE</td><td>A3</td></tr><tr><td>DRG NO</td><td> </td></tr><tr><td>REVISION</td><td> </td></tr></table>	SCALE	1:2500	SHEET SIZE	A3	DRG NO		REVISION	
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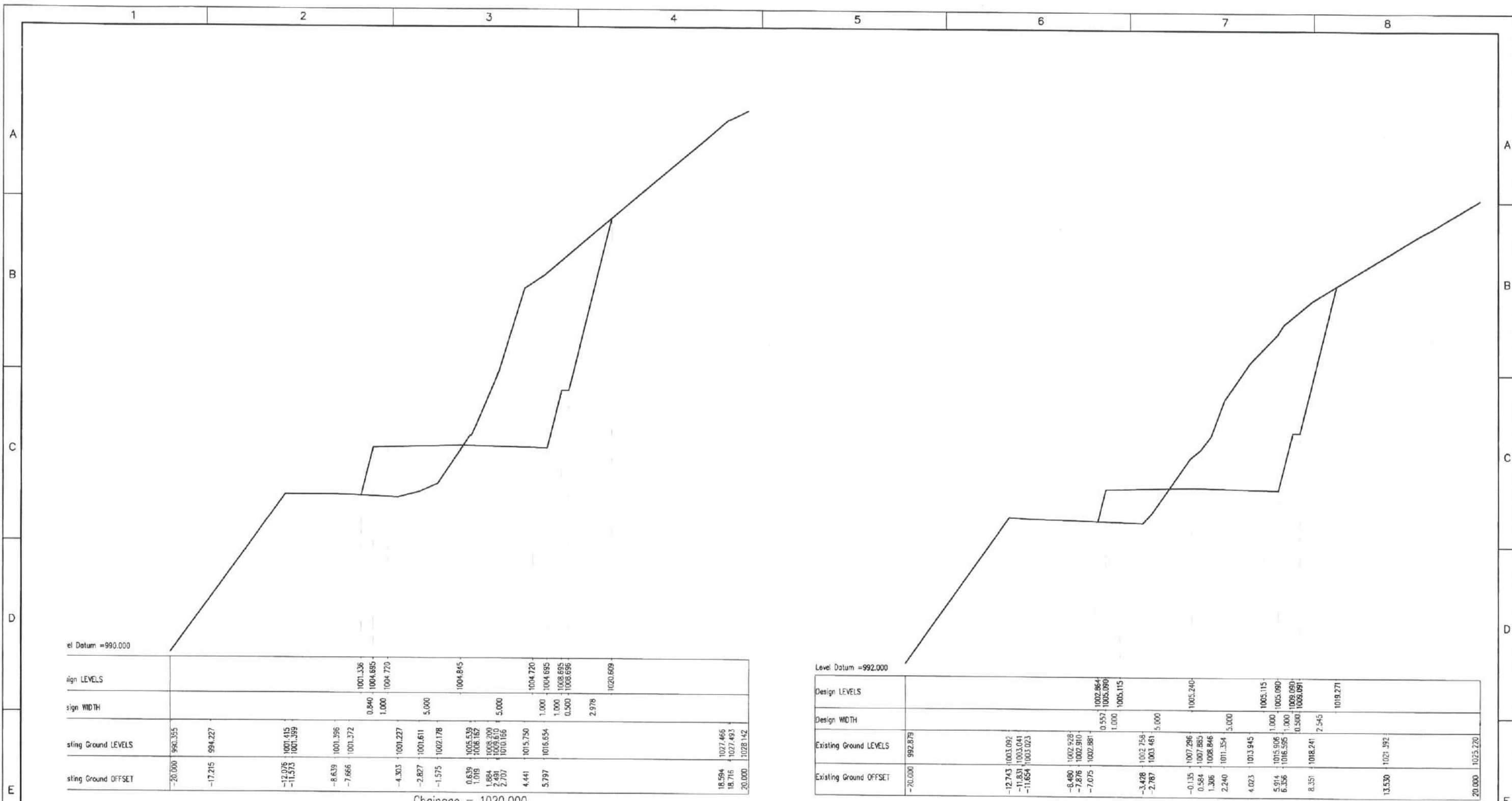






CLIENT:  National Highways and Infrastructure Development Corporation PTI Building, 4 Parliament Street, Sansad Marg Area, New Delhi, Delhi 110001	PROJECT: Construction of Bi-directional tunnel at Km 83 of length 495m including approaches road of 664m and a major bridge of 110m from Km 82 + 675 to Km 82 + 925 on NH- 244	EPC CONTRACTOR:  BEIGH CONSTRUCTION COMPANY Pvt. Ltd. Plot no : 4, Wave mall, Bypass road, Narwal Jammu email : info@bccgroup.in	DESIGN CONSULTANT:  SPACE ENGINEERS CONSORTIUM Pvt. Ltd. in association with EQUIPO DE PROSPECCIONES, S.A. Delhi 110016, New Delhi 110016, New Delhi 110016 Jammu 181001, Jammu 181001, Jammu 181001	DESIGN DIRECTOR:  K.M. MOZA	<table><tr><td>DRAWN BY</td><td>IQRA QAYOUM</td></tr><tr><td>DESIGNED BY</td><td>DANISH ALTAF</td></tr><tr><td>CHECKED BY</td><td>MEHAK MAQBOOL</td></tr><tr><td>RECOMMENDED BY</td><td>M.DAR</td></tr><tr><td>APPROVED BY</td><td>M.DAR</td></tr></table>	DRAWN BY	IQRA QAYOUM	DESIGNED BY	DANISH ALTAF	CHECKED BY	MEHAK MAQBOOL	RECOMMENDED BY	M.DAR	APPROVED BY	M.DAR	TITLE: Cross Section Details Ch: 820 & 840 SCALE 1:2500 SHEET SIZE A3 DRG NO REVISION
DRAWN BY	IQRA QAYOUM															
DESIGNED BY	DANISH ALTAF															
CHECKED BY	MEHAK MAQBOOL															
RECOMMENDED BY	M.DAR															
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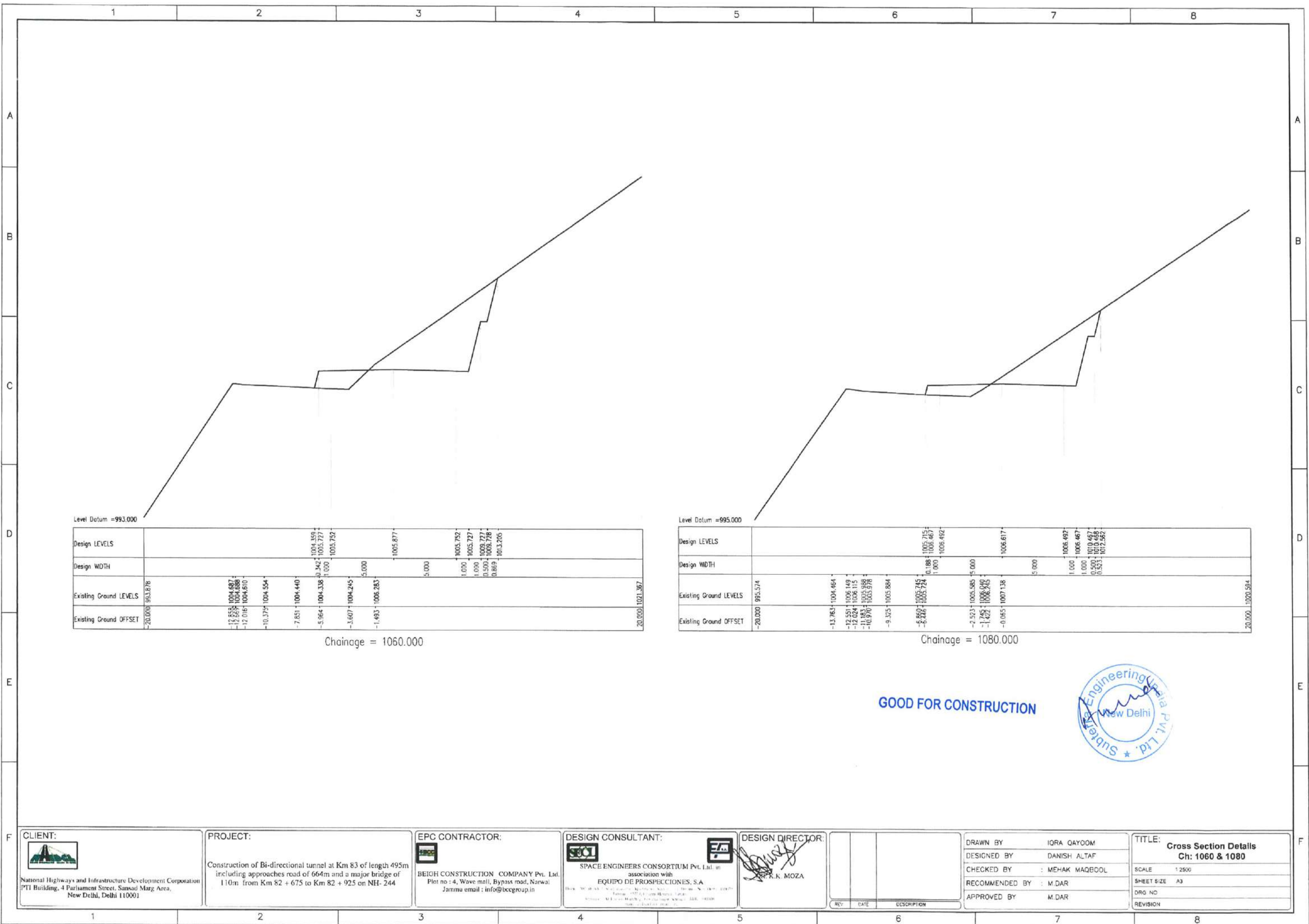


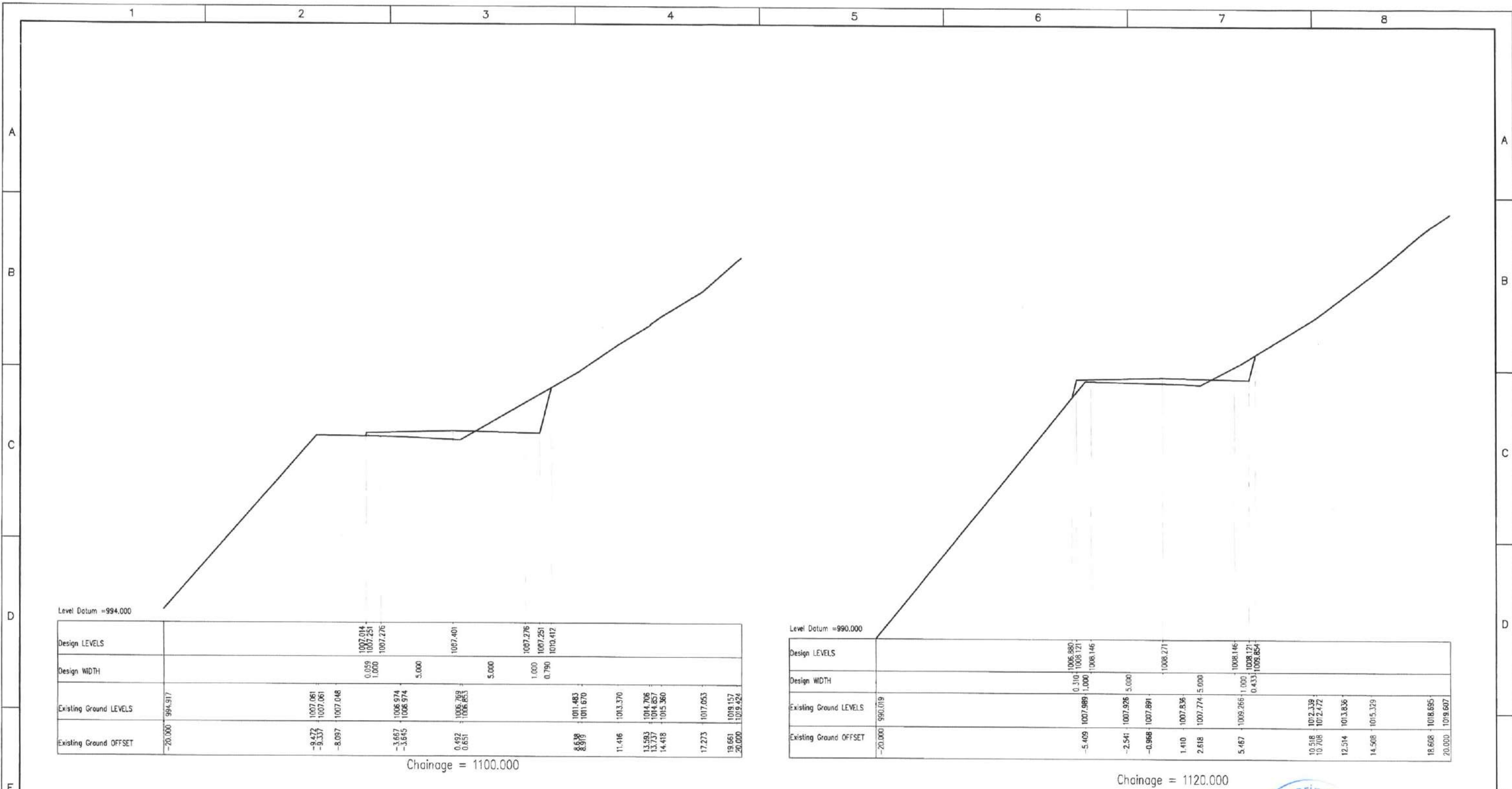


GOOD FOR CONSTRUCTION



CLIENT: National Highways and Infrastructure Development Corporation PTI Building, 4 Parliament Street, Sansad Marg Area, New Delhi, Delhi 110001	PROJECT: Construction of Bi-directional tunnel at Km 83 of length 495m including approaches road of 664m and a major bridge of 110m from Km 82 + 675 to Km 82 + 925 on NH- 244	EPC CONTRACTOR: BEIGH CONSTRUCTION COMPANY Pvt. Ltd. Plot no : 4, Wave mall, Bypass road, Narwal Jammu email : info@bccgroup.in	DESIGN CONSULTANT: SPACE ENGINEERS CONSORTIUM Pvt. Ltd. in association with EQUIPO DE PROSPECCIONES, S.A. C/O. P.O. Box 1000, San Juan, Puerto Rico 00901 Manager: Subrah Engineering Pvt. Ltd. New Delhi 110001	DESIGN DIRECTOR: SUBRAH MOZA	<table border="1"><thead><tr><th>REV</th><th>DATE</th><th>DESCRIPTION</th></tr></thead><tbody><tr><td> </td><td> </td><td> </td></tr></tbody></table>	REV	DATE	DESCRIPTION				<table border="1"><tr><td>DRAWN BY</td><td>IQRA QAYOUM</td></tr><tr><td>DESIGNED BY</td><td>DANISH ALTAH</td></tr><tr><td>CHECKED BY</td><td>MEHAK MAQBLOOL</td></tr><tr><td>RECOMMENDED BY</td><td>M.DAR</td></tr><tr><td>APPROVED BY</td><td>M.DAR</td></tr></table>	DRAWN BY	IQRA QAYOUM	DESIGNED BY	DANISH ALTAH	CHECKED BY	MEHAK MAQBLOOL	RECOMMENDED BY	M.DAR	APPROVED BY	M.DAR	<table border="1"><tr><td>TITLE:</td><td>Cross Section Details</td></tr><tr><td></td><td>Ch: 1020 & 1040</td></tr><tr><td>SCALE</td><td>1:2500</td></tr><tr><td>SHEET SIZE</td><td>A3</td></tr><tr><td>DRG NO</td><td> </td></tr><tr><td>REVISION</td><td> </td></tr></table>	TITLE:	Cross Section Details		Ch: 1020 & 1040	SCALE	1:2500	SHEET SIZE	A3	DRG NO		REVISION	
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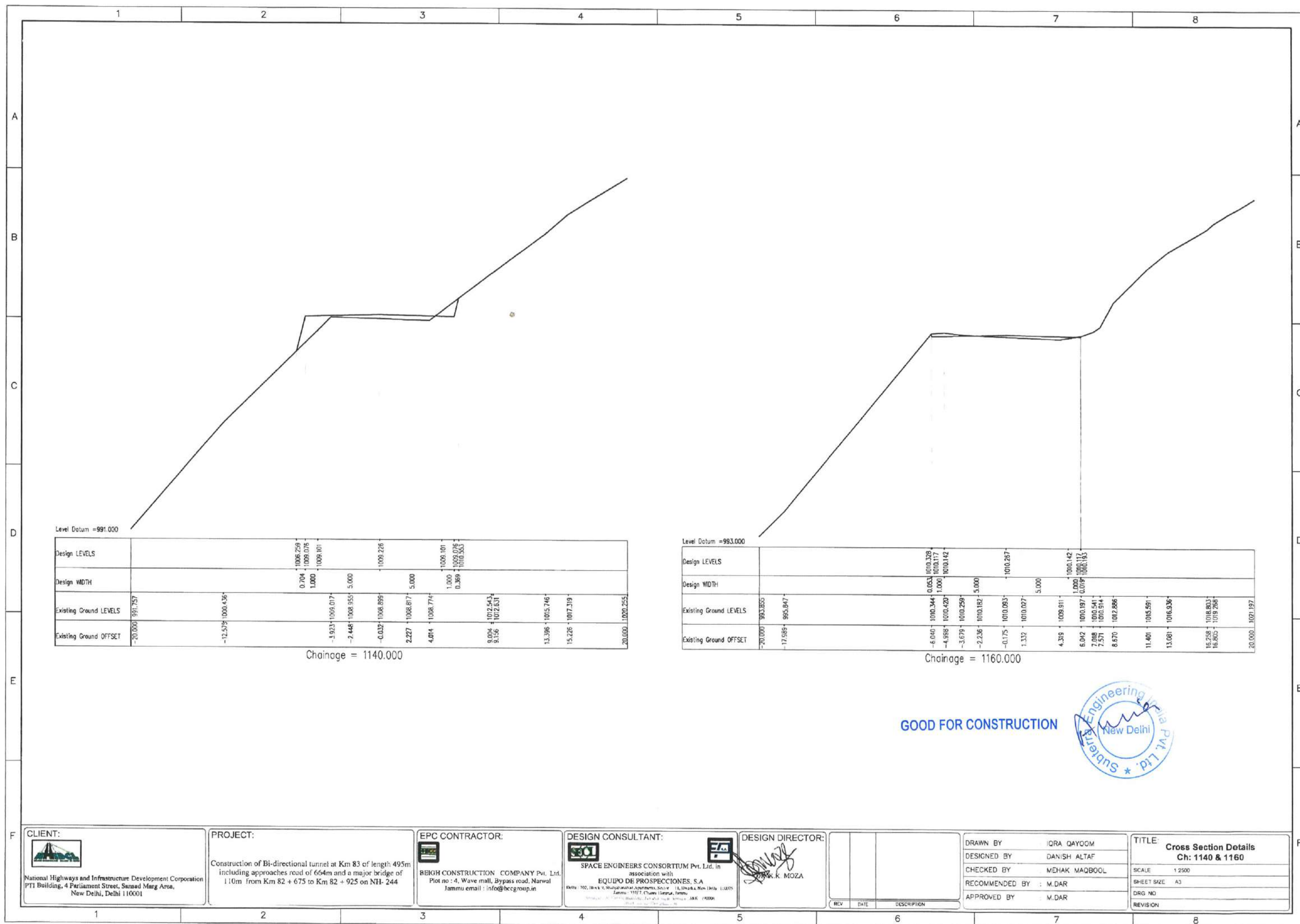




GOOD FOR CONSTRUCTION



CLIENT:  National Highways and Infrastructure Development Corporation PTI Building, 4 Parliament Street, Sansad Marg Area, New Delhi, Delhi 110001		PROJECT: Construction of Bi-directional tunnel at Km 83 of length 495m including approaches road of 664m and a major bridge of 110m from Km 82 + 675 to Km 82 + 925 on NH- 244		EPC CONTRACTOR:  BEIGH CONSTRUCTION COMPANY Pvt. Ltd. Plot no : 4, Wave mall, Bypass road, Narwal Jammu email : info@bcgroup.in		DESIGN CONSULTANT:  SPACE ENGINEERS CONSORTIUM Pvt. Ltd. in association with EQUIPO DE PROSPECCIONES, S.A. Delhi: B-2, Block-3, Mayapuri Phase II, New Delhi - 110029 Jammu: 110019, 4th Floor, Jammu, Jammu Srinagar: At Poonch Building, Jammu Road, Srinagar - 190008 email: info@seconline.com		DESIGN DIRECTOR:  K. MOZA		DRAWN BY IQRA QAYOUM		TITLE: Cross Section Details Ch: 1100 & 1120	
DESIGNED BY DANISH ALTAH		CHECKED BY MEHAK MAQBOOL		RECOMMENDED BY M.DAR		APPROVED BY M.DAR		SCALE 1:2500		SHEET SIZE A3			
REV.		DATE		DESCRIPTION		DRG NO		REVISION					



CLIENT:



National Highways and Infrastructure Development Corporation
PTI Building, 4 Parliament Street, Sansad Marg Area,
New Delhi, Delhi 110001

PROJECT:

Construction of Bi-directional tunnel at Km 83 of length 495m
including approaches road of 664m and a major bridge of
110m from Km 82 + 675 to Km 82 + 925 on NH- 244

EPC CONTRACTOR:



BEIGH CONSTRUCTION COMPANY Pvt. Ltd.
Plot no : 4, Wave mall, Bypass road, Narwal
Jammu email : info@bccgroup.in

DESIGN CONSULTANT:



SPACE ENGINEERS CONSORTIUM Pvt. Ltd. in
association with
EQUIPO DE PROSPECCIONES, S.A.
B-102, Block 7, Shalimar Garden, Sector 16, Gurgaon, New Delhi - 110075
Jammu - 191001, Phone : 01924-110075
Jammu - 191001, Phone : 01924-110075

DESIGN DIRECTOR:



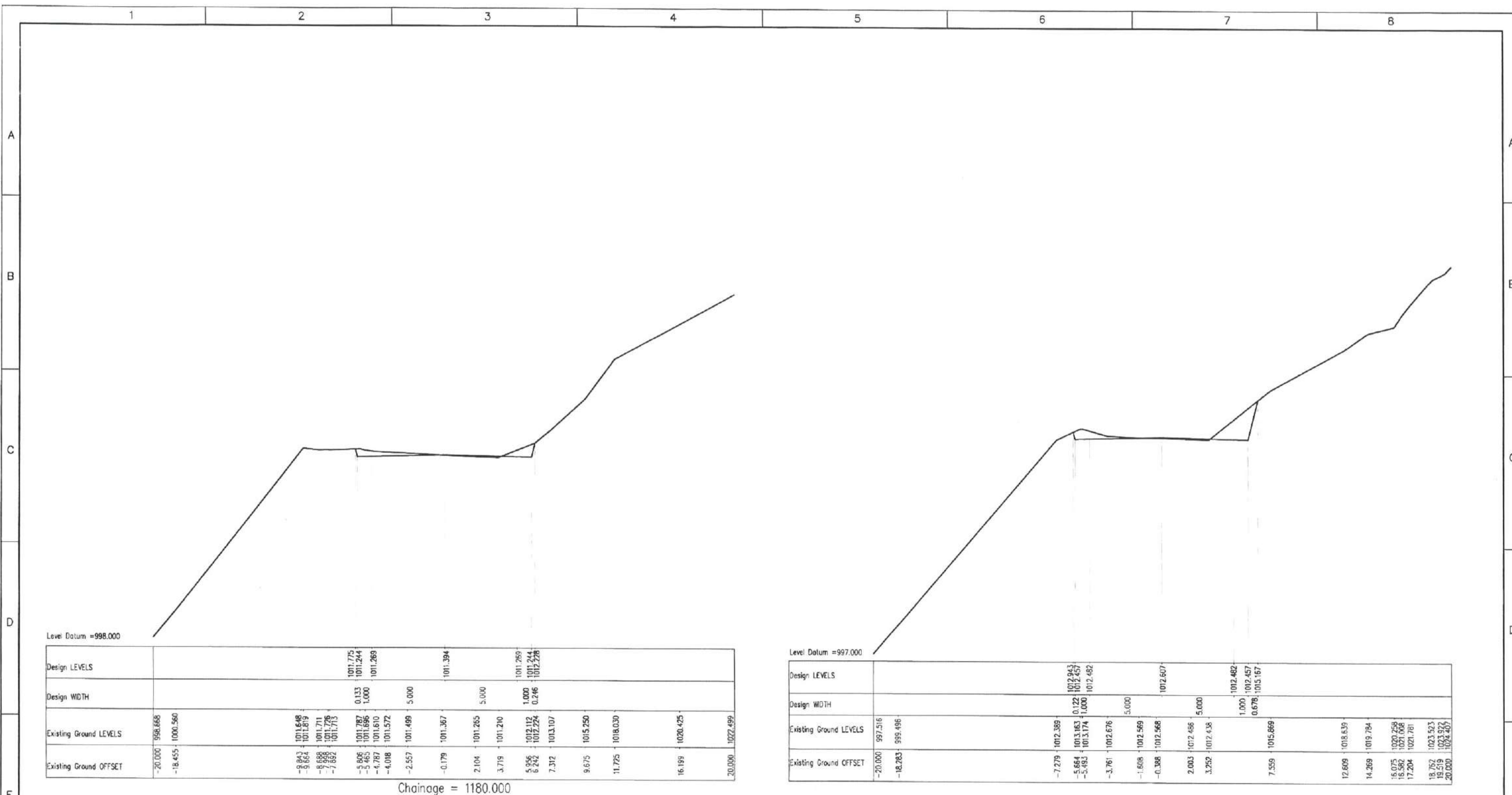
DR. K. K. MOZA

REV	DATE	DESCRIPTION

DRAWN BY : IQRA QAYOUM
DESIGNED BY : DANISH ALTAF
CHECKED BY : MEHAK MAQBOOL
RECOMMENDED BY : M.DAR
APPROVED BY : M.DAR

TITLE: Cross Section Details
Ch: 1140 & 1160

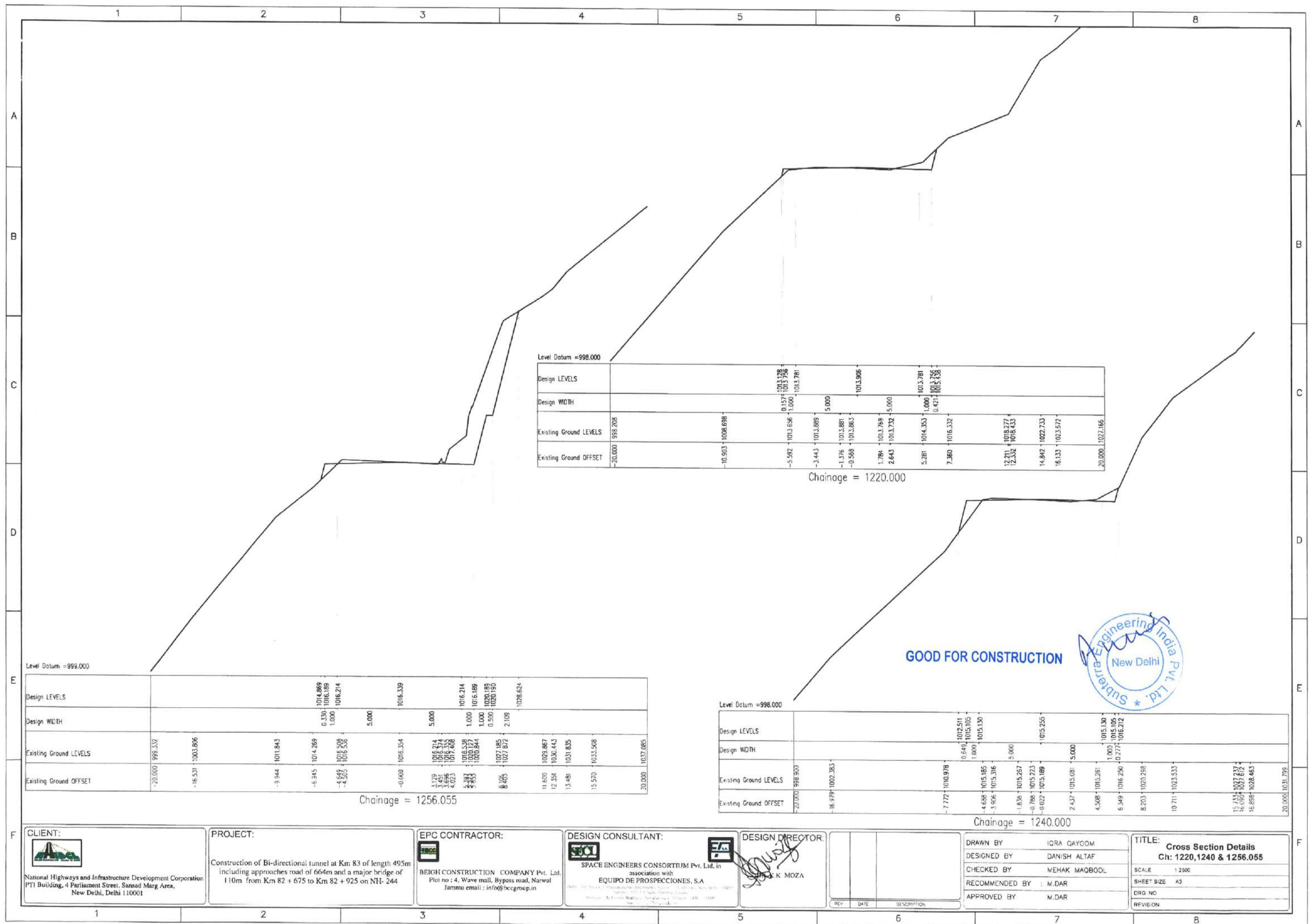
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DRG NO :
REVISION :



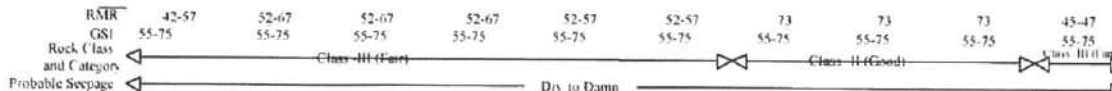
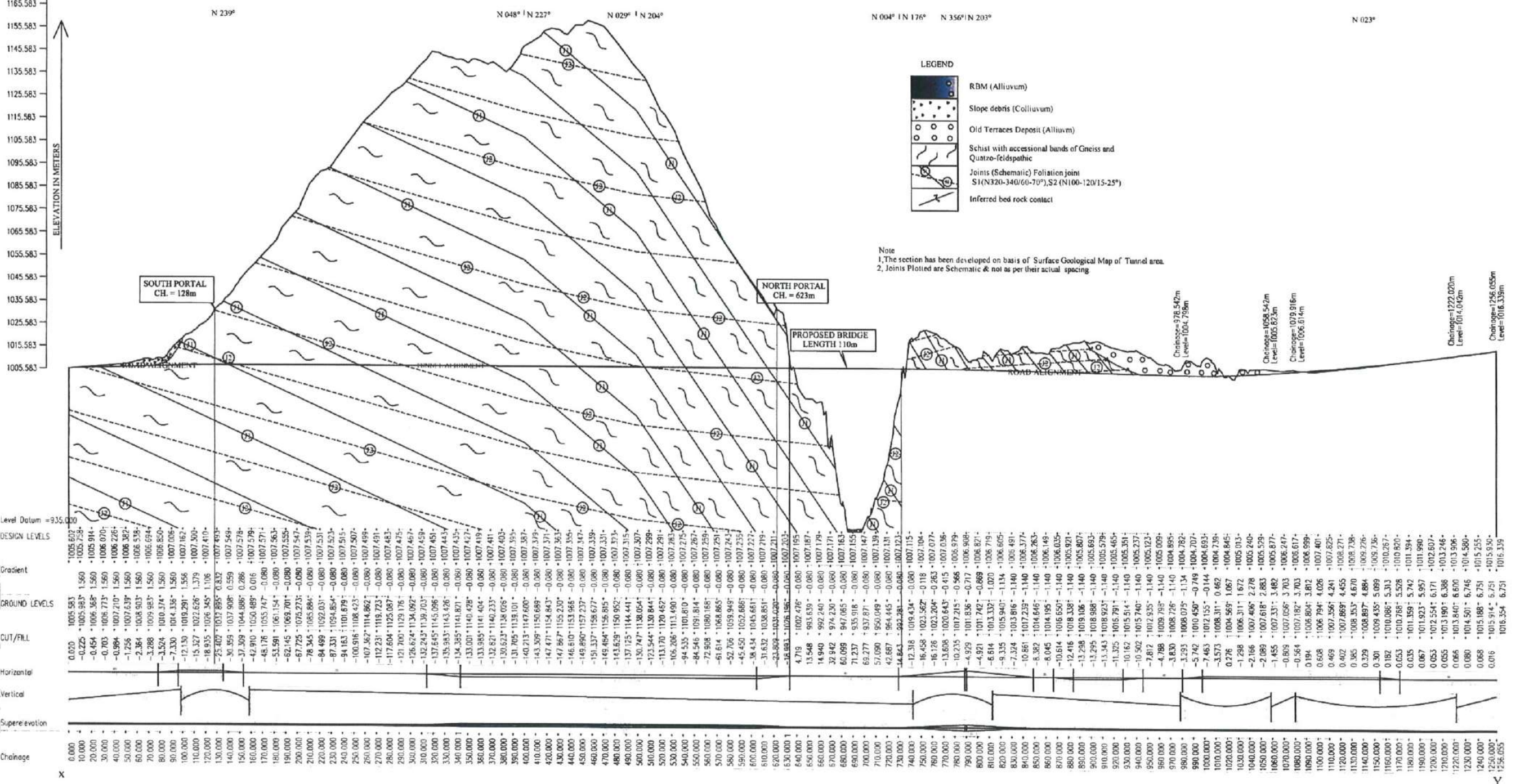
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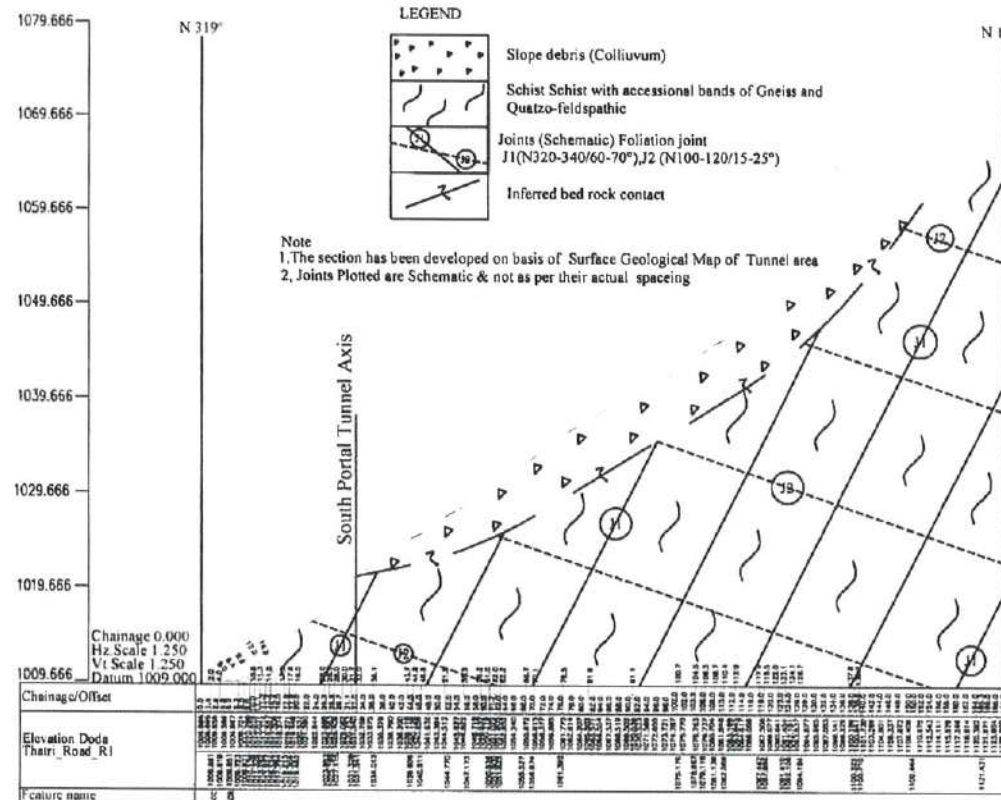
Interpreted Geological Section Along Bi-Directional Tunnel & Road from South West to North East



GOOD FOR CONSTRUCTION

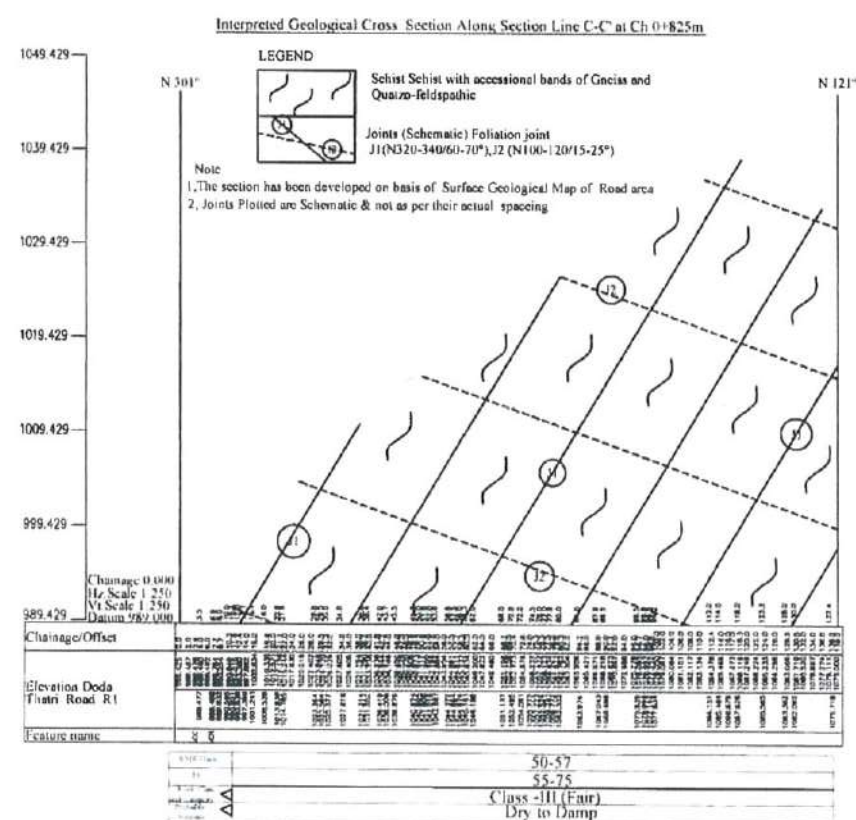


CLIENT: National Highways and Infrastructure Development Corporation PTI Building, 4 Parliament Street, Sansad Marg Area, New Delhi, Delhi 110001	PROJECT: Construction of Bi-directional tunnel at Km 83 of length 495m including approaches road of 664m and a major bridge of 110m from Km 82 + 675 to Km 82 + 925 on NH- 244	EPC CONTRACTOR: BEIGH CONSTRUCTION COMPANY Pvt. Ltd. Plot no : 4, Wave mall, Bypass road, Naawal Jammu email : info@beighgroup.in	DESIGN CONSULTANT: SPACE ENGINEERS CONSORTIUM Pvt. Ltd. in association with EQUIPO DE PROSPECCIONES, S.A. Calle 100, Block 1, Sector 1, Urbanización, Santa Fe, Bogotá, Colombia Tel: +57 1 454 4444 Fax: +57 1 454 4444 Email: info@equipo.com.co	DESIGN DIRECTOR: MOZA	DRAWN BY AMAAN DESIGNED BY FARHATUN NISA CHECKED BY JAVAID RECOMMENDED BY FARHATUN NISA APPROVED BY JAVAID	TITLE Interpreted Geological Section Along Bi-Directional Tunnel & Road from South West to North East SCALE 1:3500 SHEET SIZE A3 DRG NO REVISION
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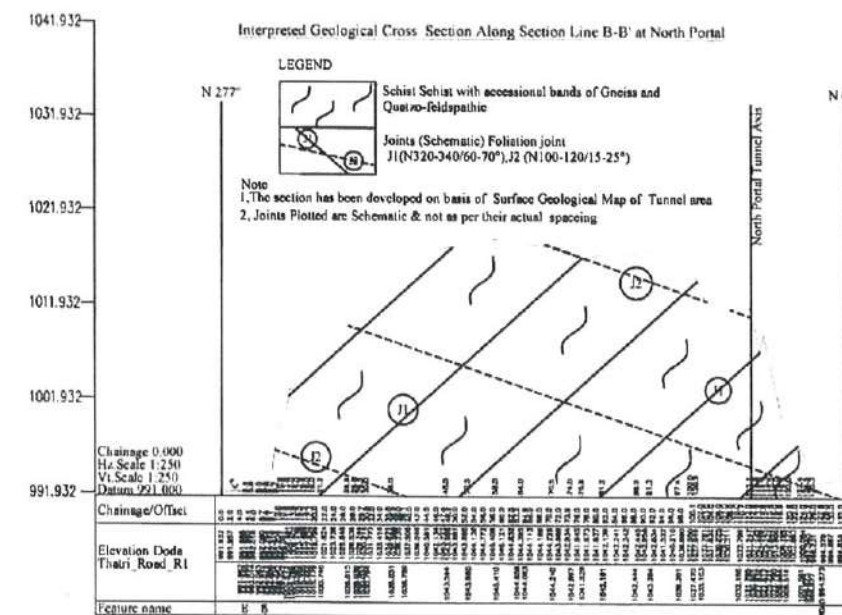


RMR Basic	52-56
GSJ	55-75
Rock Class and Category	Class -III (Fair)
Probable Seepage	Dry to Damp

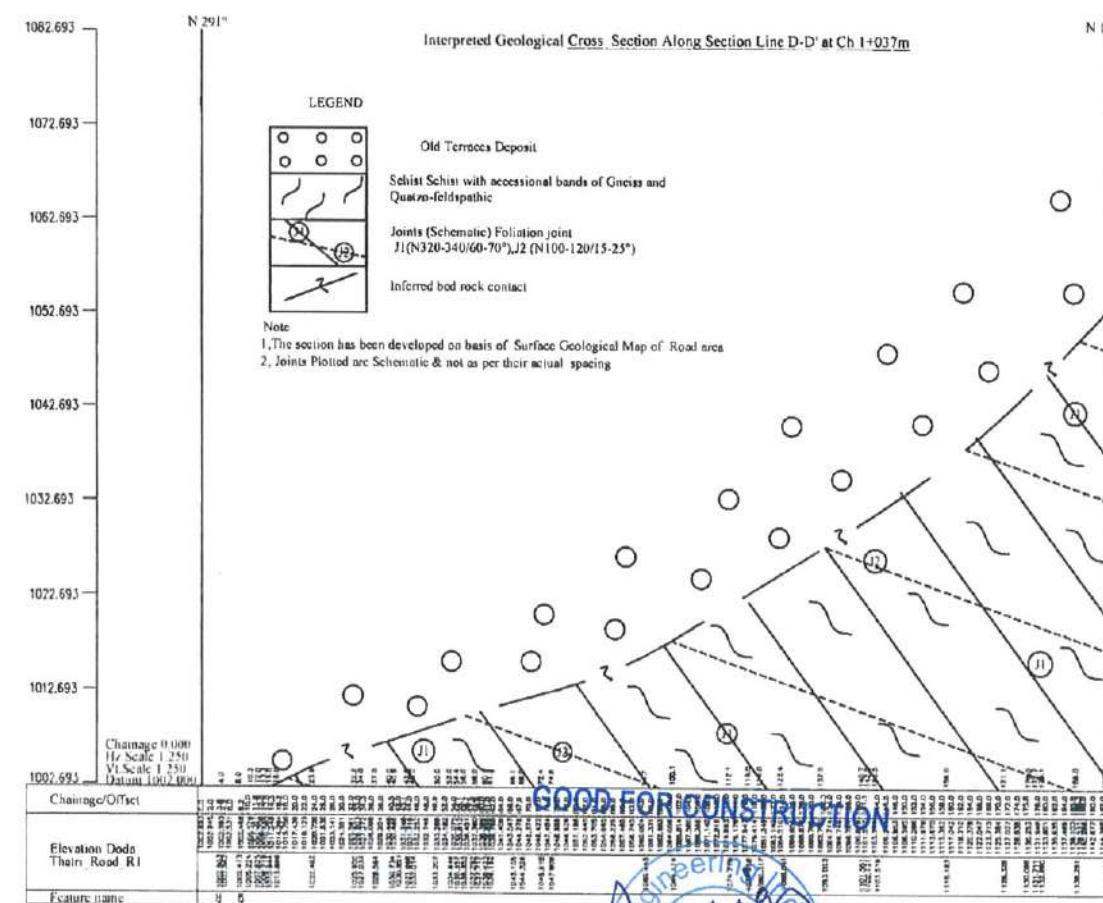
Interpreted Geological Cross Section Along Section Line A-A' at South Portal

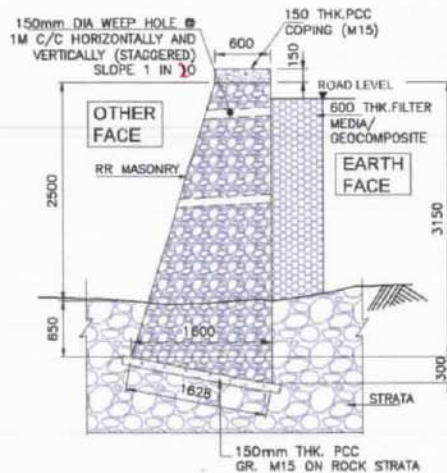


RMR Basic	50-57
GSJ	55-75
Rock Class and Category	Class -III (Fair)
Probable Seepage	Dry to Damp

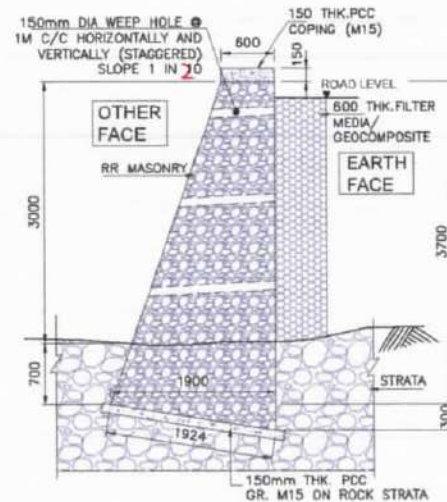


RMR Basic	50-52
GSJ	55-75
Rock Class and Category	Class -III (Fair)
Probable Seepage	Dry to Damp

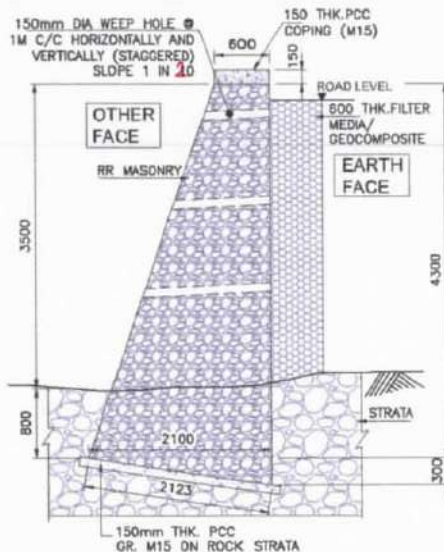




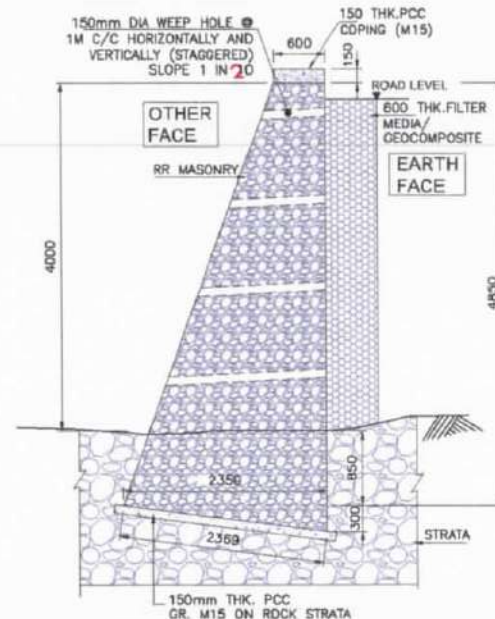
TYPICAL SECTION - 1
FOR RR, DRY MASONRY RETAINING
WALL OF HEIGHT 2.5M



TYPICAL SECTION - 2
FOR RR, DRY MASONRY RETAINING
WALL OF HEIGHT 3.0M



TYPICAL SECTION - 3
FOR RR, DRY MASONRY RETAINING
WALL OF HEIGHT 3.5M



TYPICAL SECTION - 4
FOR RR, DRY MASONRY RETAINING
WALL OF HEIGHT 4.0M

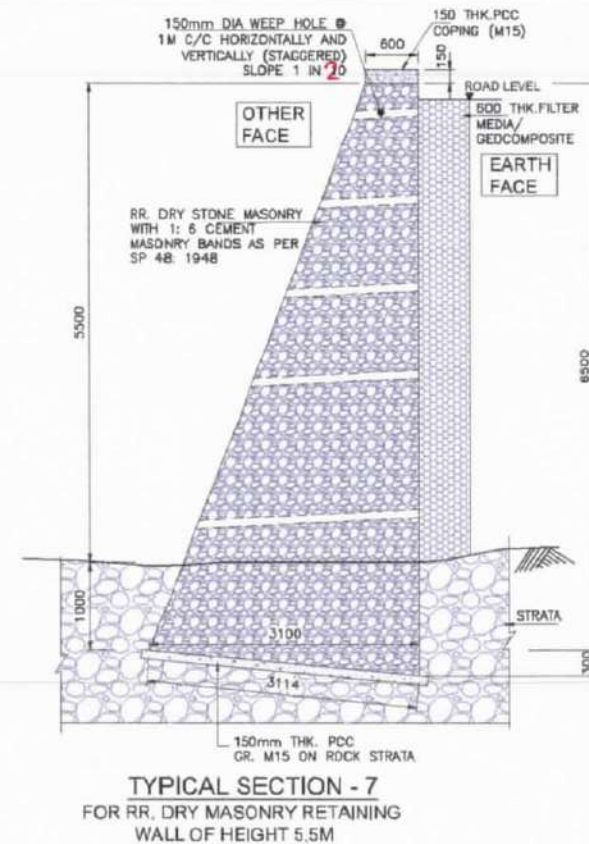
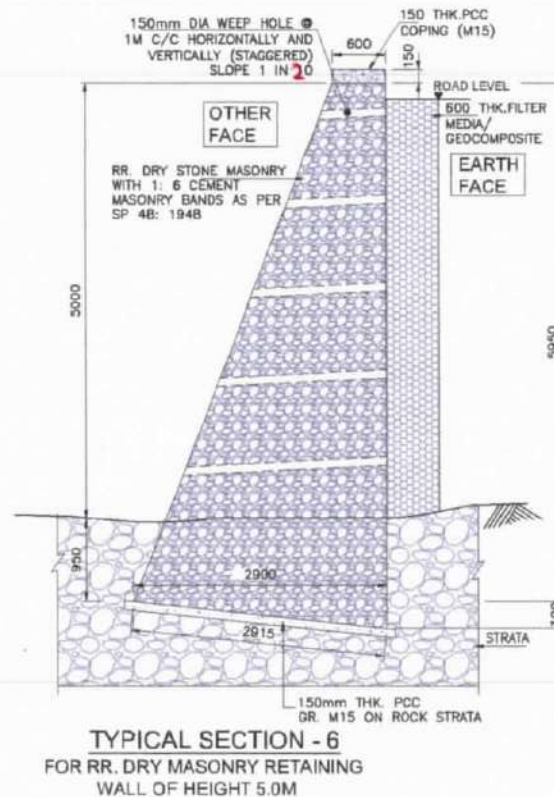
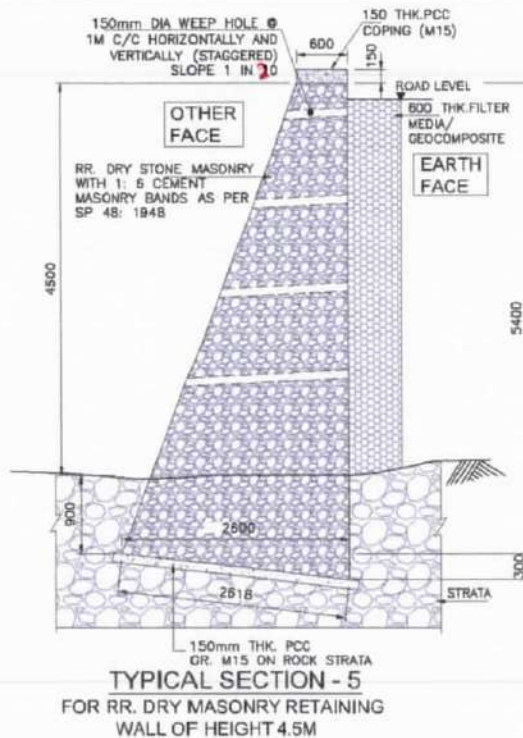
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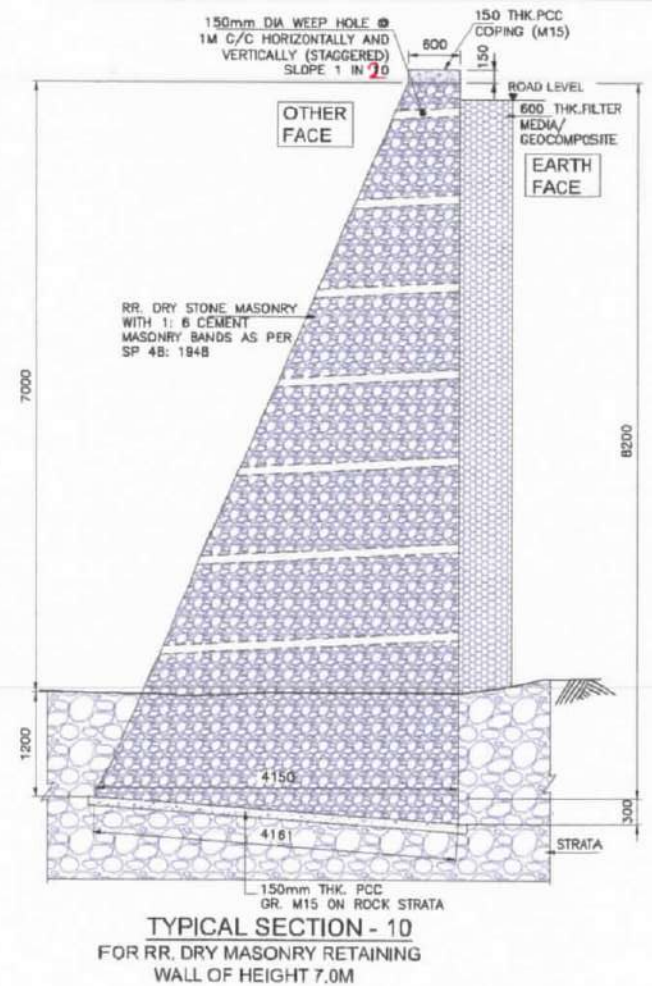
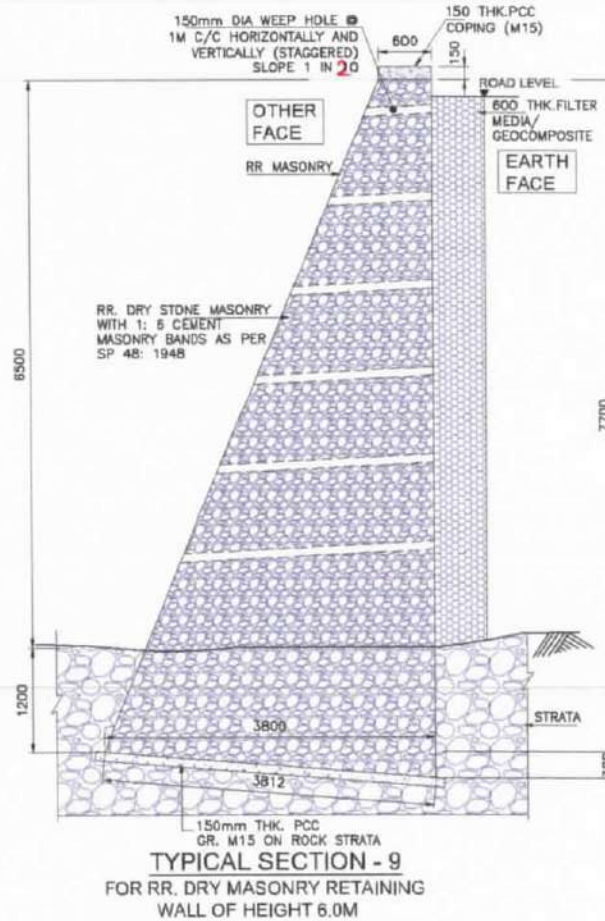
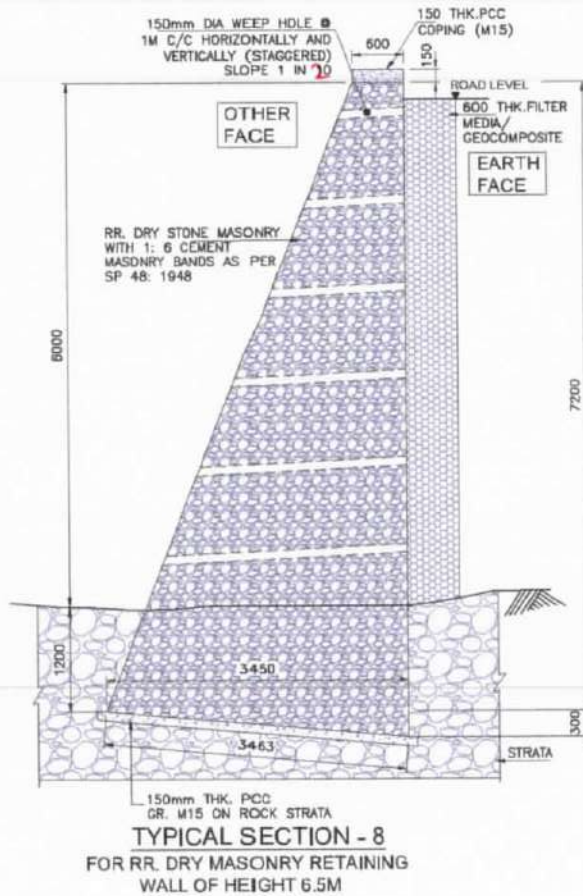
1. ALL DIMENSION ARE IN MILLIMETERS, LEVELS IN METRES AND CHAINAGES IN KILOMETERS UNLESS OTHERWISE MENTIONED.
2. THE DRAWING SHALL BE READ IN CONJUNCTION WITH RELEVANT APPROVED HIGHWAY DRAWINGS FOR STRUCTURE ORIENTATION, SKEW ANGLE, FRI & CAMBER/SUPERELEVATION ETC.
3. 150 DIA WEEP HOLES WITH SLOPE 1 IN 20 SHALL BE PROVIDED AT SPACING OF 1000 C/C BOTH HORIZONTAL AND VERTICALLY (STAGGERED) IN FULL HEIGHT OF THE RETAINING WALL WITH BOTTOM MOST ROW 150MM ABOVE G.L.
4. BACK FILLING AND GEO COMPOSITE MEMBRANE BEHIND THE RETAINING WALL SHALL CONFIRMING TO IRC:78-2014 HAVING PROPERTIES $\phi=38$, $C=0$. CONTRACTOR WILL ENSURE CAREFUL PLACEMENT OF BACKFILL AND PROPER PACKING. PROPER BONDING OF MASONRY COURSES AS PER SPECIFICATIONS OF IRC:SP. 48-1998.
5. CEMENT MORTAR (1:3) IS TO BE GIVEN FOR COPING.
6. THE REQUIRED SAFE BEARING CAPACITY IS 20 T/SQM, WHICH SHOULD BE CHECKED AND VERIFIED AT SITE BEFORE EXECUTION OF WORK. IF ANY VARIATION NOTICED, IT SHOULD BE INFORMED TO THE DESIGN CONSULTANT.
7. MINIMUM EDGE AT TOE END SHALL BE MINIMUM 900MM FROM TOE AS PER IS 1904

RR MASONRY SPECIFICATIONS (AS PER IS 1597(I):1992)

1. **SIZE:** THE LENGTH OF STONE SHALL NOT EXCEED THREE TIMES THE HEIGHT AND BREADTH ON BASE SHALL NOT BE GREATER THAN THREE FOURTH OF THICKNESS OF WALL NOR LESS THAN 150mm, THE HEIGHT OF STONE FOR RUBBLE MASONRY MAY BE UP TO 300mm.
2. **MATERIAL:** ALL STONES USED SHALL BE OF STRONG HARD AND DURABLE. ALSO IF SHOULD BE FREE FROM DEFECTS LIKE CAVITIES, CRACKS, FLAWS, SAND HOLES, THE PERCENTAGE OF WATER ABSORPTION NOT EXCEED 5 PERCENT.
3. **CONSTRUCTION OPERATIONS:** THE STONE SHALL BE LAID SO THAT THE PRESSURE IS ALWAYS PERPENDICULAR TO THE NATURAL BED. STONES SHALL BE SUFFICIENTLY WETTED BEFORE LAYING TO PREVENT ABSORPTION OF WATER FROM MORTAR. VERTICAL JOINTS SHALL BE STAGGERED AS FAR AS POSSIBLE. BELL SHAPED BOND STONES AS HEADERS SHALL NOT BE USED. AT ALL ANGULAR JUNCTIONS THE STONES AT EACH ALTERNATE COURSE SHALL BE WELL BONDED INTO RESPECTIVE COURSES OF THE ADJACENT WALL.







Dwg no.: BCC/NH/DCL/EPC/RRM/01

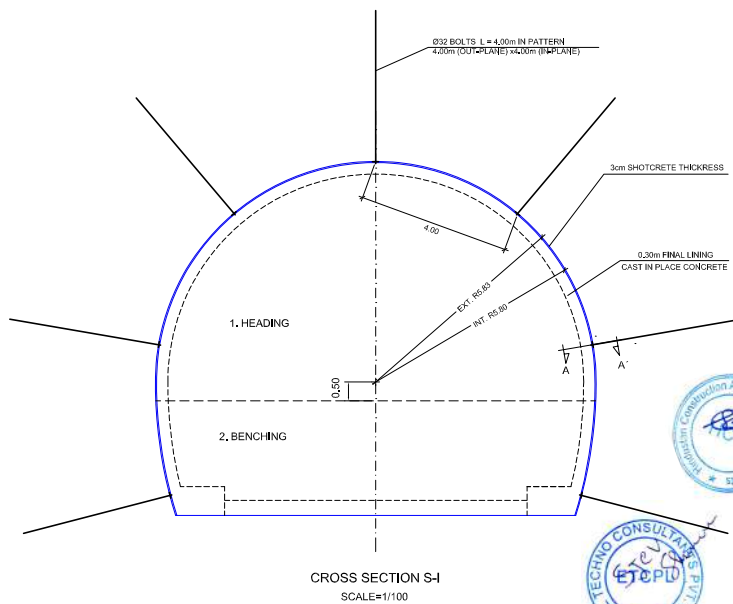
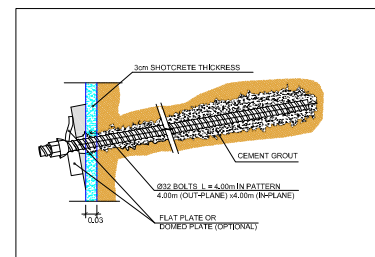


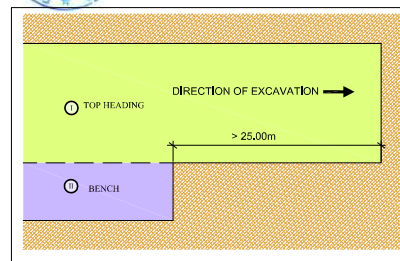
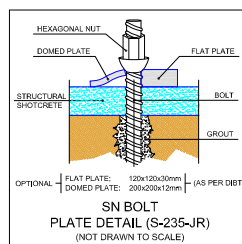
TABLE OF MATERIALS		
ELEMENT	TYPE	CONTROL LEVEL
SHOTCRETE	M30 GRADE	USUAL
BOLTS	FRICTION BOLT (40mm breaking load 270kN)	USUAL
	BOLTS 032 Fe 5000	
POLYPROPYLENE FIBER	SEE NOTE	USUAL
CAST IN PLACE CONCRETE IN FOOTING AND WALLS	M30 GRADE	USUAL

FIBERS AMOUNT WILL BE ENOUGH WHEN THE ENERGY ABSORPTION EXCEEDS 750 Joules IN THE PLATE TEST.

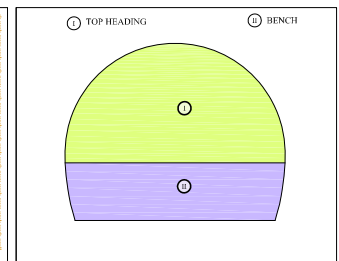


SECTION A-A
SCALE=1/10

- NOTE 1:
-PAYMENT LINE OF EXCAVATION DEFINES THE TECHNICAL AND GEOLOGICAL OVERBREAK.
-FILLING OF THE OVERBREAK WILL BE PAID WITH THE PRICE OF m³ OF SHOTCRETE.
- NOTE 2:
-MAX. TOP-HEADING EXCAVATION ROUND LENGTH: 4.00m.
-MAX. BENCH EXCAVATION ROUND LENGTH: TWICE TO THREE TIMES THE TOP-HEADING.
-THE EXCAVATION ROUND MAY BE REDUCED ACCORDING TO AUSCULTATION RESULTS AND SEEING.

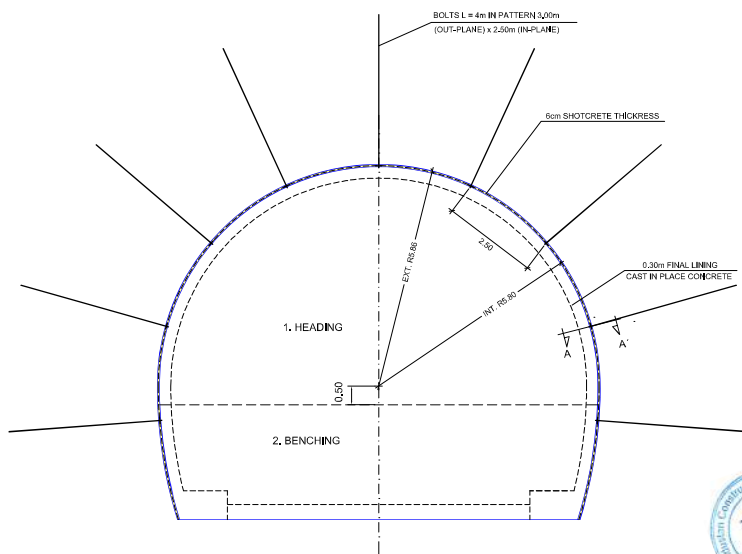


SCHEME EXCAVATION OF LONGITUDINAL SECTION



EXCAVATION OF CROSS SECTION S-I

CLIENT  National Highways and Infrastructure Development Corporation P71 Building, 4 Parliament Street, Sansad Marg Area, New Delhi, Delhi 110001	PROJECT Construction Of Bi-Directional tunnel at km 83 of length 495m building approaches road of 604 m and a major bridge of 110 m from km 82 +475 to km 82 +525 on NH-244	EPC CONSULTANT  Belg Construction Company Pvt.Ltd Plot no 4, Wave mall, Bypass road, Naraina, Jammu email: info@belggroup.in	DESIGN CONSULTANT  Space Engineers Consortium Pvt.Ltd In associated with Equipo De Prospecciones, S.A	R2 02-02-22 R1 24-05-21 R0 27-02-21 FINAL DESIGN FINAL DESIGN	DRAWN BY: CARMEN PEREZ GERRANO DESIGNED BY: DIANA CASTANEDO DE LUCAS CHECKED BY: PROF. FCO. CASTANEDO NAVARRO RECOMMENDED BY: DIANA CASTANEDO LUCAS APPROVED BY: PROF. FCO. CASTANEDO NAVARRO SCALE: INDICATED	FOR TYPICAL SECTIONS EXCAVATION AND SUPPORT (CROSS SECTION I) PROJ. K08T/STES/INDR2/1/5 R2
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CROSS SECTION S-II
SCALE=1/100

NOTE 1:

-PAYMENT LINE OF EXCAVATION DEFINES THE TECHNICAL AND GEOLOGICAL OVERBREAK.

-FILLING OF THE OVERBREAK WILL BE PAID WITH THE PRICE OF m³ OF SHOTCRETE.

NOTE 2:

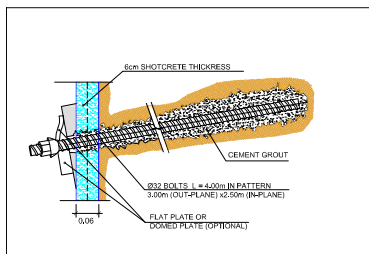
-MAX. TOP HEADING EXCAVATION ROUND LENGTH: 2.00m.

-MAX. BENCH EXCAVATION ROUND LENGTH: TWICE TO THREE TIMES THE TOP HEADING.

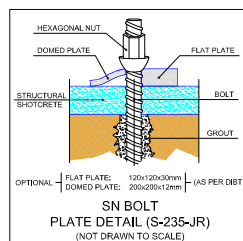
-THE EXCAVATION ROUND MAY BE REDUCED ACCORDING TO AUSCULTATION RESULTS AND SEINGS.

TABLE OF MATERIALS		
ELEMENT	TYPE	CONTROL LEVEL
SHOTCRETE	M30 GRADE	USUAL
BOLTS	FRICION BOLT (Minimum Breaking Load 42000N)	USUAL
	BOLTS Ø32 Fe 500D	
POLYPROPYLENE FIBER	SEE NOTE	USUAL
CAST IN PLACE CONCRETE IN FOOTING AND WALLS	M30 GRADE	USUAL

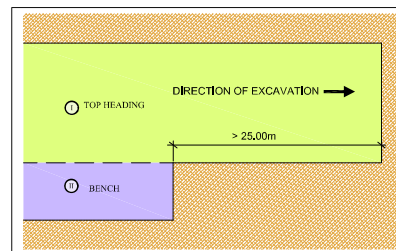
FIBERS AMOUNT WILL BE ENOUGH WHEN THE ENERGY ABSORPTION EXCEEDS 300 JOULES IN THE PLATE TEST.



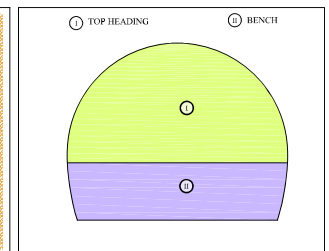
SECTION A-A
SCALE=1/10



SN BOLT
PLATE DETAIL (S-235-JR)
(NOT DRAWN TO SCALE)



SCHEME EXCAVATION OF LONGITUDINAL SECTION



EXCAVATION OF CROSS SECTION S-II

CLIENT



National Highways and Infrastructure
Development Corporation
P71 Building, 4 Parliament Street, Sansad Marg Area,
New Delhi, Delhi 110001

PROJECT

Construction Of Bi-Directional Tunnel at
km 83 of length 495m including approaches
road, of 684 m and a major bridge of 110 m from
km 82 +475 to km 82 +525 on NH-244

EPC CONSULTANT



Beigh Construction Company Pvt.Ltd
Plot no 4, Wave mall, Bypass road,
Nareed, Jamnagar
email: info@bccgroup.in

DESIGN CONSULTANT



Space Engineers Consortium Pvt.Ltd
In associated with
Equipo De Prospecciones, S.A

R2
R1
R0

02-02-22
24-05-21
27-03-21

FINAL DESIGN
FINAL DESIGN
FINAL DESIGN

DRAWN BY: CARMEN PEREZ SERRANO

DESIGNED BY: DIANA CASTANEDO DE LUCAS

CHECKED BY: PROF. FCO. CASTANEDO NAVARRO

RECOMMENDED BY: DIANA CASTANEDO LUCAS

APPROVED BY: PROF. FCO. CASTANEDO NAVARRO

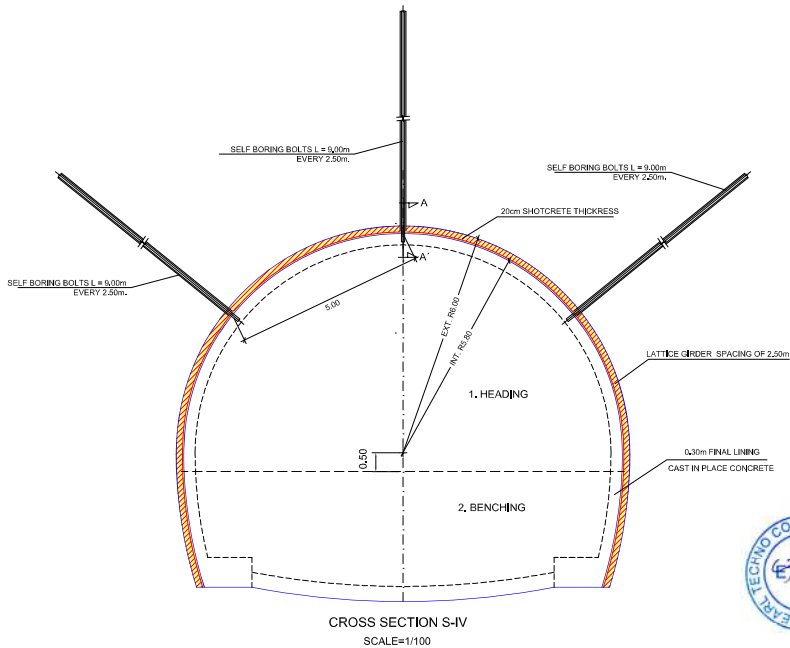
SCALE: INDICATED

FIG. 1
TYPICAL SECTION
EXCAVATION AND SUPPORT
(CROSS SECTION II)

PROJ. NO.
K08T/STES/IND/R2/0/5

DATE:
R2

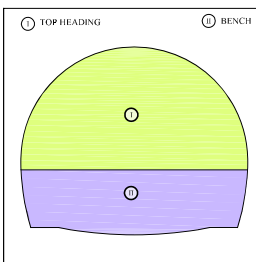
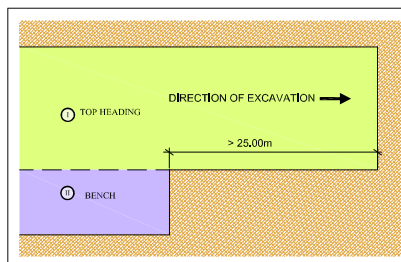
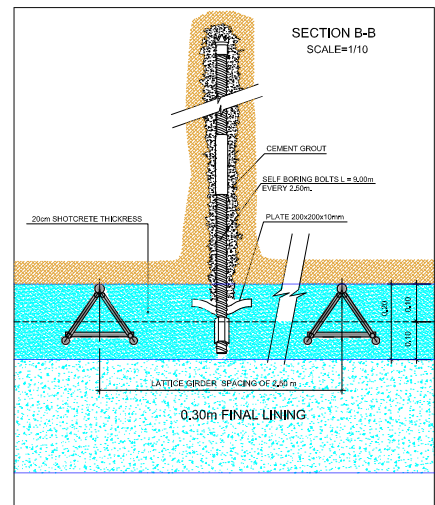
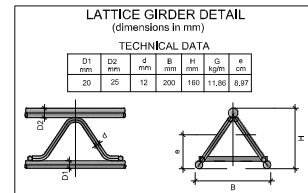




NOTE 1:
-PAYMENT LINE OF EXCAVATION DEFINES THE TECHNICAL AND GEOLOGICAL OVERBREAK.

NOTE 2:
-MAX. TOP-HEADING EXCAVATION ROUND LENGTH: 2.50m.
-MAX. BENCH EXCAVATION ROUND LENGTH: TWICE TO THREE TIMES THE TOP-HEADING.
-THE EXCAVATION ROUND MAY BE REDUCED ACCORDING TO AUSCULTATION RESULTS AND SEEING.

TABLE OF MATERIALS		
ELEMENT	TYPE	CONTROL LEVEL
SHOTCRETE	M30 GRADE	USUAL
BOLTS	FRICTION BOLT (Minimum Breaking Load 2200N)	USUAL
	BOLTS Ø32 Fx 5000	
POLYPROPYLENE FIBER	SEE NOTE	USUAL
CAST IN PLACE CONCRETE IN FOOTINGS AND WALLS	M30 GRADE	USUAL



CLIENT  National Highways and Infrastructure Development Corporation P7 Building, 4 Parliament Street, Sansad Marg Area, New Delhi-110001	PROJECT Construction Of Bi-Directional tunnel at km 83 of length 495m including approaches road, of 684 m and a major bridge of 110 m from km 82 +575 to km 82 +525 on NH-244	EPC CONSULTANT  Belgh Construction Company Pvt.Ltd Plot no 4, Wave mall, Bypass road, Naraina, New Delhi email: info@bccgroup.in	DESIGN CONSULTANT  Space Engineers Consortium Pvt.Ltd In associated with Equipo De Prospecciones, S.A	FINAL DESIGN R2 02-02-22 R1 24-05-21 R0 27-03-21 FINAL DESIGN FINAL DESIGN SCALE: INDICATED	DESIGNED BY: CARMEN PEREZ SERRANO CHECKED BY: PROF. COCA CASTAÑEDO NAVARRO RECOMMENDED BY: DIANA CASTAÑEDO LUCAS APPROVED BY: PROF. COCA CASTAÑEDO NAVARRO SCALE: INDICATED	TYPICAL SECTION EXCAVATION AND SUPPORT (CROSS SECTION IV) R2
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